3 MEMORY AVE, CROOKWELL - PROPOSED 20 LOT SUBDIVISION PROJECT:

CONCEPT CIVIL ENGINEERING PLANS PLANSET:

CLIENT: **BLUE SOX DEVELOPMENTS**



LOCALITY PLAN NOT TO SCALE

LGA: UPPER LACHLAN SHIRE COUNCIL

3 MEMORY AVE CROOKWELL NSW 2583 LOT 2/DP702788

PS01-B100	C	STAGING PLAN
PS01-B310	E	SOIL AND WATER MANAGEMENTAND DEMOLITION PLAN (STAGE 1)
PS01-B320	D	SOIL AND WATER MANAGEMENTAND DEMOLITION PLAN (STAGE 2)
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PS01-B340	C	SOIL AND WATER MANAGEMENT DETAILS - RUSLE CALCULATIONS
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PS01-C100	C	EARTHWORKS GRADING PLAN (ULTIMATE DESIGN)
PS01-C110	C	EARTHWORKS GRADING PLAN (STAGE 1)
PS01-C120	C	EARTHWORKS GRADING PLAN (STAGE 2)
PS01-C500	C	EARTHWORKS CUT-FILL PLAN (ULTIMATE DESIGN)
PS01-C510	D	EARTHWORKS CUT-FILL PLAN (STAGE 1)
PS01-C520	С	EARTHWORKS CUT-FILL PLAN (STAGE 2)
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PS01-C610	В	EARTHWORKS SITE SECTIONS (SHEET 2)
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ROADWOR	KS	
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PS01-D110	D	ROADWORKS PLAN (STAGE 1)
PS01-D120	D	ROADWORKS PLAN (STAGE 2)
PS01-D200	C	ROAD 1 (21-MRC01) LONGITUDINAL SECTION AND TYPICAL SECTIONS (SHEET 1)
PS01-D210	C	ROAD 1 (21-MRC01) LONGITUDINAL SECTION AND TYPICAL SECTIONS (SHEET 2)
PS01-DZ00	C	COMBINED SERVICES TRENCH AND DRIVEWAY GUIDELINES DETAILS
DRAINAGE		
PS01-E100	C	DRAINAGE PLAN (ULTIMATE DESIGN)
PS01-E110	С	DRAINAGE PLAN (STAGE 1)
PS01-E120	С	DRAINAGE PLAN (STAGE 2)
PS01-E200	С	DRAINAGE DETAILS
PS01-E300	C	DRAINAGE LONGITUDINAL SECTIONS AND PIT SCHEDULES (SHEET 1)
PS01-E310	С	DRAINAGE LONGITUDINAL SECTIONS AND PIT SCHEDULES (SHEET 2)
PS01-E320	С	DRAINAGE LONGITUDINAL SECTION AND PIT SCHEDULES (SHEET 3)
PS01-E600	С	OSD CATCHMENT PLAN, MODEL AND RESULTS
STRUCTUR	E AND	PAVEMENTS
PS01-G400	С	SIGNAGE, LINEMARKING & STREETSCAPE PLAN

DEVELOPMENT OVERVIEW PLAN

PS01-A300 D SUBDIVISION LAYOUT PLAN CONSTRUCTION MANAGEMENT WORKS

DRAWING LIST DWG NO. REV DWG TITLE

GENERAL

REV DESCRIPTION	DATE	DRA	.WN DESI	SIGNED CH	HECKED /	APPRVD
F MINOR AMENDMENTS	28/05/2		н с		AVG	GT
E MINOR AMENDMENTS	24/04/2	025 SS	H F	RK	AVG	GT
D MINOR AMENDMENT	25/03/2	025 SS	H RL	L/PC	AVG	GT
C MINOR AMENDMENTS	20/03/2	025 SS	H RL	L/PC	AVG	GT
B MINOR AMENDMENTS	11/03/2	025 SS	H RL	L/PC	AVG	GT
A INITIAL RELEASE	25/02/2	025 SS	H RL	L/PC	AVG	GT

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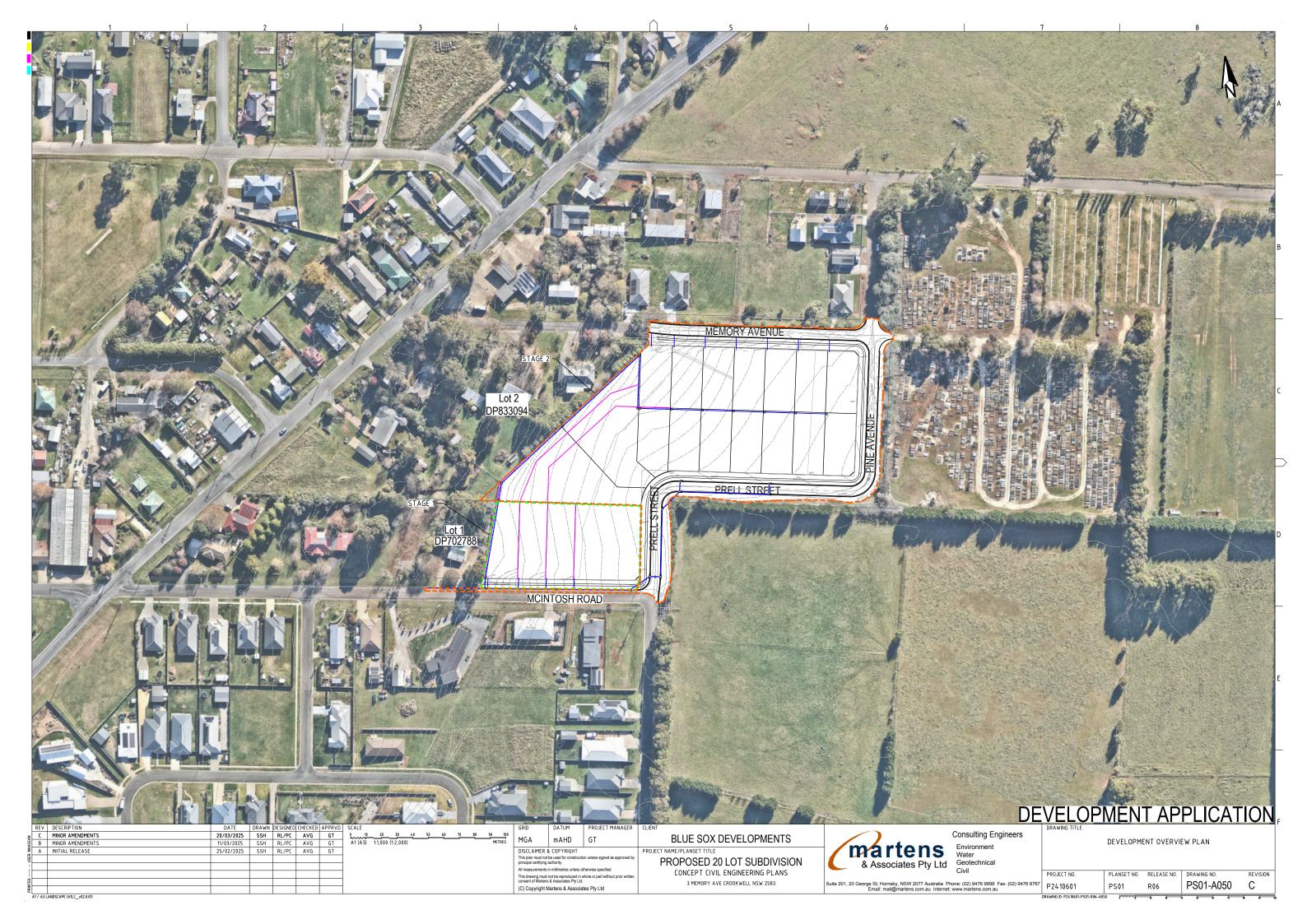
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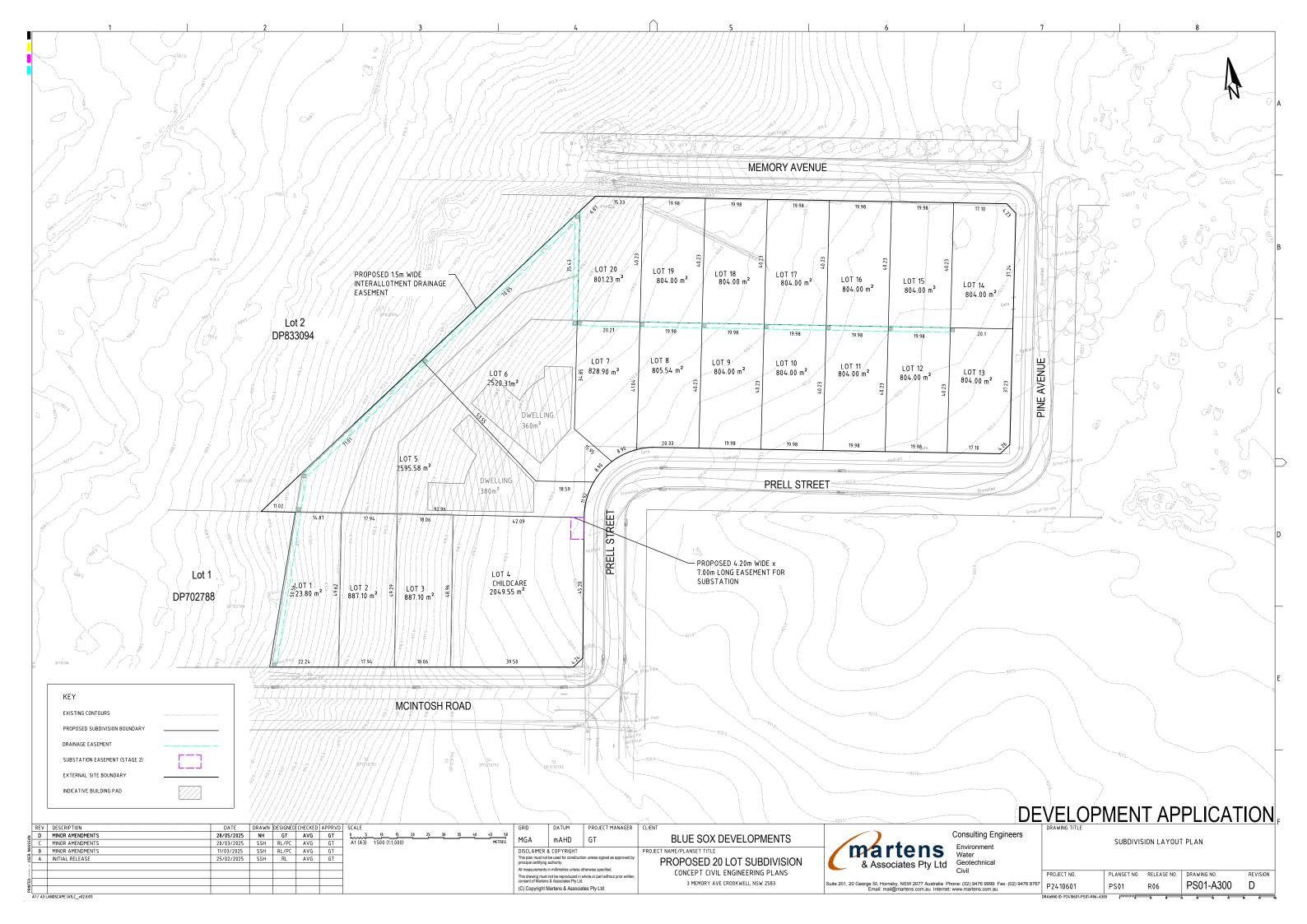
Consulting Engineers

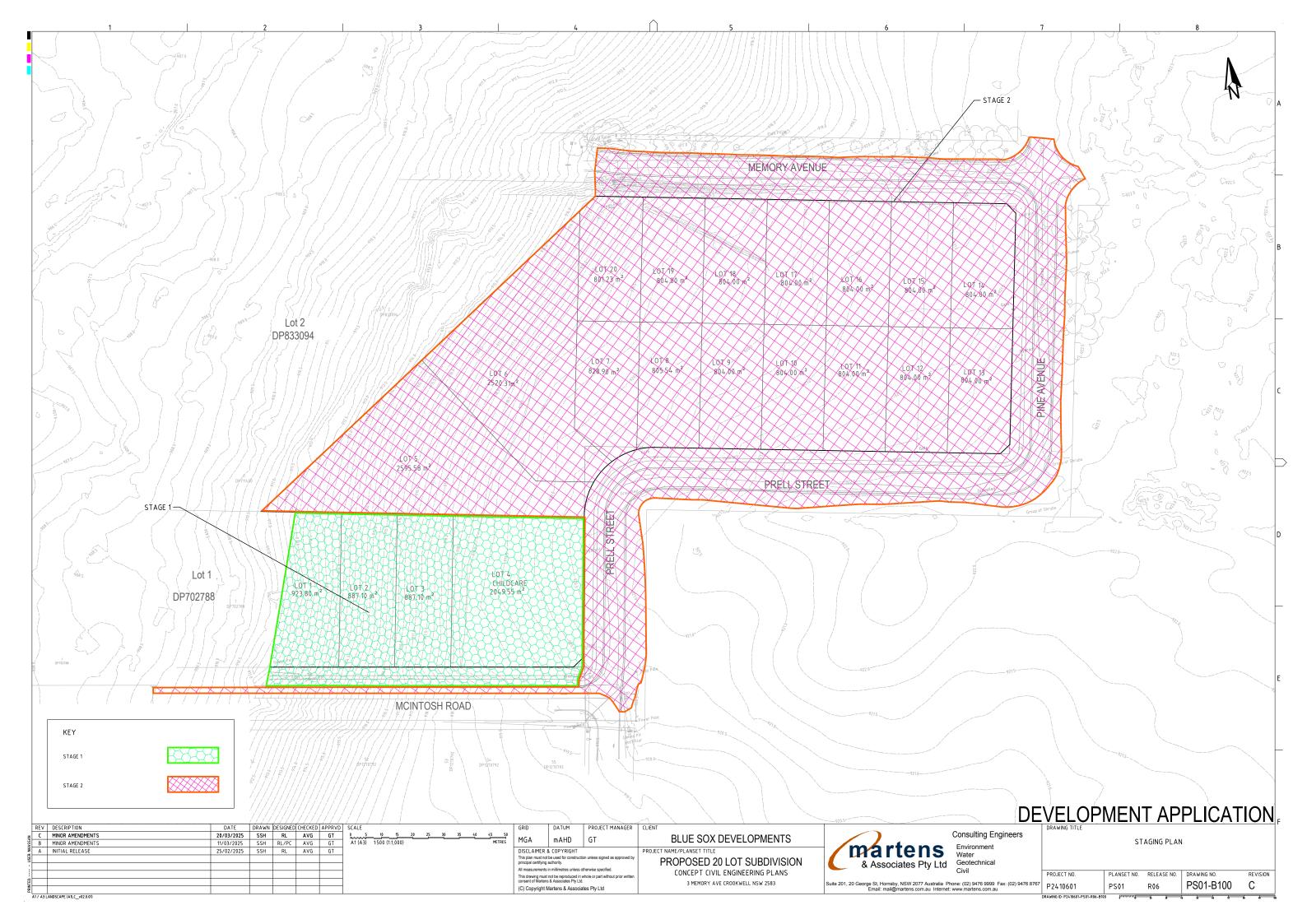
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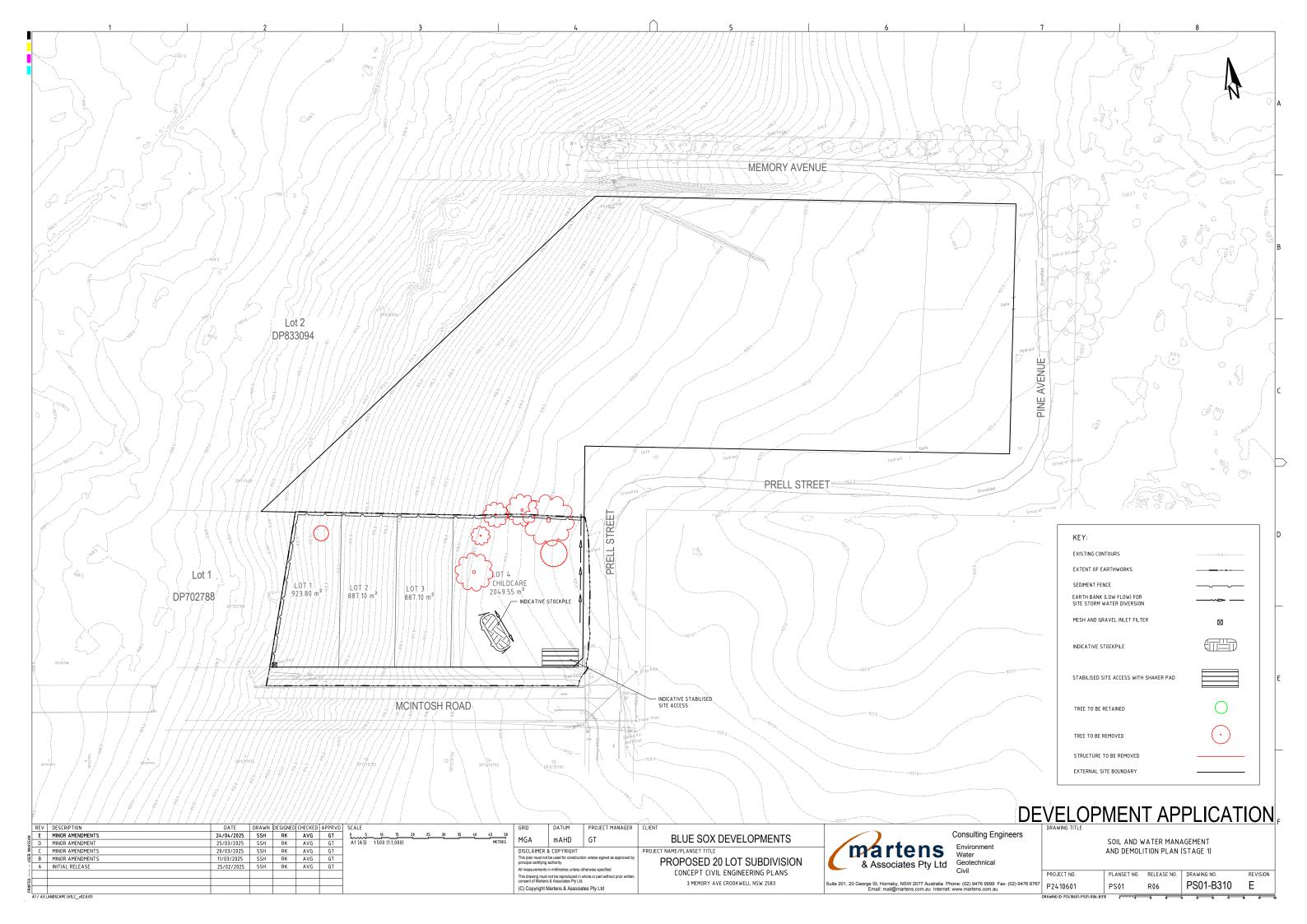
DEVELOPMENT APPLICATION

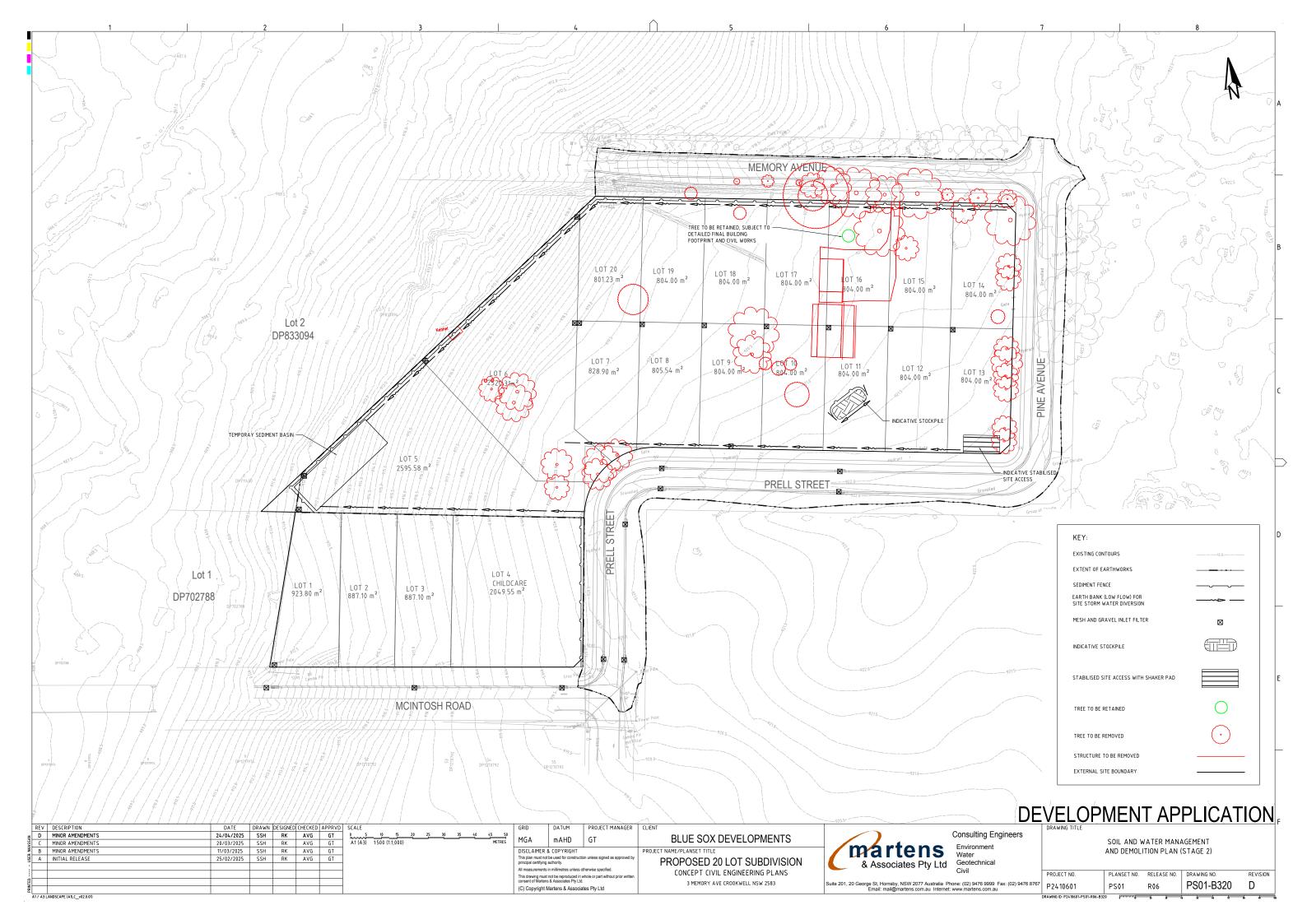
P2410601









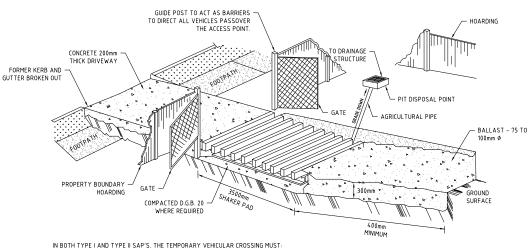


STABILISED ACCESS POINT

TYPE II SAP

THE TYPE II SAP DESIGN IS MORE DEFINED IN THAT IT REQUIRES AN AREA OF BALLAST WITHIN THE SITE COMBINED WITH A SHAKER PAD: ADJACENT THE SHAKER PAD AND IN THE PUBLIC WAY IS A TEMPORARY (CONCRETE) VEHICULAR CROSSING. (SEE DIAGRAM)

STABILISED ACCESS POINT - TYPE 2



- CONNECT TO AN EXISTING GUTTER LAYBACK (WHERE THE KERB AND GUTTER EXIST) . IF A GUTTER LAYBACK DOES NOT EXIST THEN THE CONNECTION MUST BE MADE TO THE GUTTER BY REMOVING THE ADJICENT KERB SECTION ONLY
- CONNECT TO A DISH CROSSING (WHERE KERB AND GUTTER DOES NOT EXIST). IF A DISH CROSSING DOES NOT EXIST, THEN IT MUST BE

CONSTRUCTED IN ACCORDANCE WITH DETAILS CONTAINED IN COUNCIL'S ISSUED FOOTPATH CROSSING LEVELS

IT SHOULD BE NOTED THAT THESE TYPES OF SAPS ARE CONSIDERED TO BE APPLICABLE FOR THE MAJORITY OF ACTIVITIES HOWEVER SOME SITES MAY REQUIRE SPECIAL CONSIDERATION.

SHAKER PAD (CATTLE GRID)

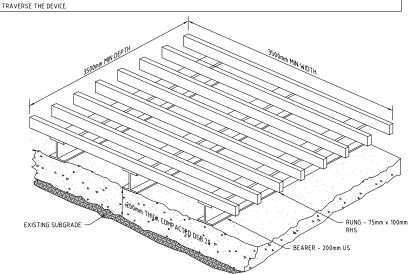
A CORRECTLY DESIGNED AND INSTALLED SHAKER PAD WILL ASSIST IN PREVENTING SEDIMENT TRANSFERE FROM A SITE. ANY STABILISED ACCESS POINT (SAP) CAN BE DESIGNED WITH A SHAKER PAD (COMPULSOPRY IN TYPE II SAP'S)

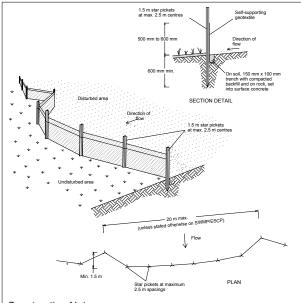
SHAKER PADS CAN BE DESIGNED AND CONSTRUCTED TO ENABLE RE-USE ON FUTURE PROJECTS.

THE SHAKER PAD

- . MUST BE DESIGNED AND CERTIFIED BY A PRACTICING STRUCTURAL ENGINEER. THE CERTIFIED DESIGN SHOULD BE SUBMITTED WITH THE RELEVENT APPLICATION.
- CAN BE CONSTRUCTED FROM ANY SUITABLE MATERIAL.
- MUST BE LOCATED ON A SUITABLY PREPARED AND COMPACTED SUB-GRADE/BASE MATERIAL
- MUST BE SITUATED SUCH THAT THE RUNGS OF THE SHAKER PAD ARE LEVEL WITH THE ADJOINING NATURAL SURFACE.
- MUST BE A MINIMUM OF 3.5m IN LENGTH. MUST BE A MINIMUN OF 3.5m IN WIDTH.
- MUST HAVE CLEAR SPACING BETWEEN RUNGS OF 200 250mm.
- RUNGS MUST HAVE A MAXIMUM WIDTH (BEARING AREA) OF 75mm
- MUST HAVE A MINIMUM CLEAR DEPTH OF 300mm IE FORM THE ROP OF THE RUNG TO THE FINISHED SUB-GRADE/BASE LEVEL.

THE SHAKER PAD MUST BE PROVIDED WITH SUITABLE BARRIERS AT THE SIDES TO ENSURE THAT ALL TYERS OF VEHICLES LEAVING THE SITE





Construction Notes

- COTIST DICTION NOTES

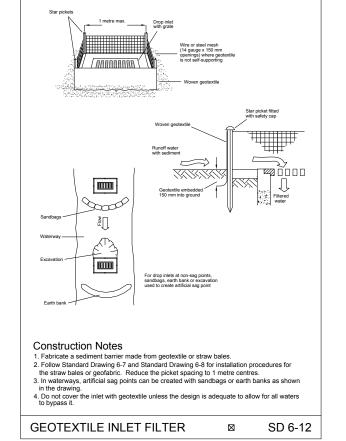
 1. Construct sediment fences as close as possible to being parallel to the contours of the site, but with small returns as shown in the drawing to limit the catchment area of any one section. The catchment area should be small enough to limit water flow if concentrated at one point to 50 litres per second in the design storm event, usually the 10-year event.

 2. Cut a 150-mm deep trench along the upslope line of the fence for the bottom of the fabric to be extremed.
- be entrenched.

 3. Drive 1.5 metre long star pickets into ground at 2.5 metre intervals (max) at the downslope edge of the trench. Ensure any star pickets are fitted with safety caps.

 4. Fix self-supporting geotextile to the upslope side of the posts ensuring it goes to the base of the trench. Fix the geotextile with wire ties or as recommended by the manufacturer. Only use geotextile specifically produced for sediment fencing. The use of shade cloth for this
- purpose is not satisfactory
- Join sections of fabric at a support post with a 150-mm overlap.
 Backfill the trench over the base of the fabric and compact it thoroughly over the geotextile.

SEDIMENT FENCE SD 6-8



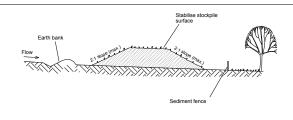
All batter grades 2(H):1(V) max. Gradient of drain Direction of flow 300 mm NOTE: Only to be used as temporary bank where maximum upslope length is 80 metres Construction Notes 1. Build with gradients between 1 percent and 5 percent. 2. Avoid removing trees and shrubs if possible - work around them 3. Ensure the structures are free of projections or other irregularities that could impede water flow. 4. Build the drains with circular, parabolic or trapezoidal cross sections, not V

- snapeu.

 5. Ensure the banks are properly compacted to prevent failure.

 6. Complete permanent or temporary stabilisation within 10 days of construction.

EARTH BANK (LOW FLOW) —> — SD 5-5



Construction Notes

- Place stockpiles more than 2 (preferably 5) metres from existing vegetation, concentrated water flow, roads and hazard areas.
 Construct on the contour as low, flat, elongated mounds.
 Where there is sufficient area, topsoil stockpiles shall be less than 2 metres in height.

- 4. Where they are to be in place for more than 10 days, stabilise following the approved ESCP or SWMP to reduce the C-factor to less than 0.10.

 5. Construct earth banks (Standard Drawing 5-5) on the upslope side to divert water around stockpiles and sediment fences (Standard Drawing 6-8) 1 to 2 metres downslope.

STOCKPILES

SD 4-1

	REV	DESCRIPTION	DATE	DRAWN	DESIGNED	CHECKED	APPRVD	SCAL
z	С	MINOR AMENDMENTS	20/03/2025	SSH	RK	AVG	GT	
ASS.	В	MINOR AMENDMENTS	11/03/2025	SSH	RK	AVG	GT	
USER: NHASSAN	Α	INITIAL RELEASE	25/02/2025	SSH	RK	AVG	GT	
USE								
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PROJECT MANAGER

DATUM

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BLUE SOX DEVELOPMENTS

PROPOSED 20 LOT SUBDIVISION CONCEPT CIVIL ENGINEERING PLANS 3 MEMORY AVE CROOKWELL NSW 2583



Consulting Engineers Environment Water

uite 201, 20 George St, Hornsby, NSW 2077 Australia Phone: (02) 9476 9999 Fax: (02) 9476 8767 Email: mail@martens.com.au Internet: www.martens.com.au

/ELOP	MEN	I AP	PLICA	MOIL
DRAWING TITLE				
SO	IL AND WATE	ER MANAGEM	ENT DETAILS	
DO IECT NO	DI ANCET NO	DELEACE NO	DD ALVING NO	DEVICION

PS01-B330 С P2410601 PS01 R06

SWMP Commentary, Detailed Calculations

Note: These "Detailed Calculation" spreadsheets relate only to high erosion hazard lands as identified in figure 4.6 or where the designer chooses to use the RUSLE to size sediment basins. The "Standard Calculation" spreadsheets should be used on low erosion hazard lands as identified by figure 4.6 and where the designer chooses not to run the RUSLE in calculations.

1. Site Data Sheet

Site Name: 3 MEMORY AVE CROOKWELL NSW 2583

Site Location: 3 MEMORY AVE CROOKWELL NSW 2583

Precinct: CROOKWELL

Description of Site: STAGE 1

Site area		S	Remarks	
Site area	CAT 1			Remarks
Total catchment area (ha)	0.54			
Disturbed catchment area (ha)	0.54			

Soil analysis									
% sand (faction 0.02 to 2.00 mm	Soil texture should be assessed through								
% silt (fraction 0.002 to 0.02 mm)	30						mechanical dispersion only. Dispersing		
% clay (fraction finer than 0.002 mm)	25						agents (e.g. Calgon) should not be used		
Dispersion percentage	10.0						E.g. enter 10 for dispersion of 10%		
% of whole soil dispersible	4						See Section 6.3.3(e)		
Soil Texture Group	С						See Section 6.3.3(c), (d) and (e)		

Rainfall data										
Design rainfall depth (days)	5						See Sections 6.3.4 (d) and (e)			
Design rainfall depth (percentile)	75						See Sections 6.3.4 (f) and (g)			
x-day, y-percentile rainfall event	14.2						See Section 6.3.4 (h)			
Rainfall intensity: 2-year, 6-hour storm	6.07						See IFD chart for the site			

RUSLE Factors										
Rainfall erosivity (R-factor)	1040					Automatic calculation from above data				
Soil erodibility (K-factor)	0.05									
Slope length (m)	100					1				
Slope gradient (%)	9					RUSLE data can be obtained from				
Length/gradient (LS-factor)	2.82					Appendixes A, B and C				
Erosion control practice (P-factor)	1.3					1				
Ground cover (C-factor)	1					1				

Ground cover (C-factor)	1			
Calculations				
Soil loss (t/ha/yr)	191			
Soil Loss Class	2			See Section 4.4.2(b)
Soil loss (m3/ha/yr)	147			
Sediment basin storage volume, m ³	13			See Sections 6.3.4(i) and 6.3.5 (e)

P2410601JS02V01 RUSLE-Stage 1

SWMP Commentary, Detailed Calculations

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1. Site Data Sheet

Site Name: 3 MEMORY AVE CROOKWELL NSW 2583

Site Location: 3 MEMORY AVE CROOKWELL NSW 2583

Precinct: CROOKWELL

Description of Site: STAGE 2

Site area		Si	Remarks	
Site area	CAT 1			Remarks
Total catchment area (ha)	2.36			
Disturbed catchment area (ha)	2.36			

Soil analysis									
% sand (faction 0.02 to 2.00 mm	55						Soil texture should be assessed through		
% silt (fraction 0.002 to 0.02 mm)	30						mechanical dispersion only. Dispersing		
% clay (fraction finer than 0.002 mm)	25						agents (e.g. Calgon) should not be used		
Dispersion percentage	10.0						E.g. enter 10 for dispersion of 10%		
% of whole soil dispersible	4						See Section 6.3.3(e)		
Soil Texture Group	С						See Section 6.3.3(c), (d) and (e)		

Rainfall data				
Design rainfall depth (days)	5			See Sections 6.3.4 (d) and (e)
Design rainfall depth (percentile)	75			See Sections 6.3.4 (f) and (g)
x-day, y-percentile rainfall event	14.2			See Section 6.3.4 (h)
Rainfall intensity: 2-year, 6-hour storm	6.07			See IFD chart for the site

RUSLE Factors				
Rainfall erosivity (R-factor)	1040			Automatic calculation from above data
Soil erodibility (K-factor)	0.05			
Slope length (m)	150			1
Slope gradient (%)	7			RUSLE data can be obtained from
Length/gradient (LS-factor)	2.605			Appendixes A, B and C
Erosion control practice (P-factor)	1.3			1
Ground cover (C-factor)	1			1

Calculations								
Soil loss (t/ha/yr)	176							
Soil Loss Class	2						See Section 4.4.2(b)	
Soil loss (m³/ha/yr)	135							
Sediment basin storage volume, m3	54						See Sections 6.3.4(i) and 6.3.5 (e)	

SWMP Commentary, Detailed Calculations

2. Storm Flow Calculations

Peak flow is given by the Rational Formula:

Qy = 0.00278 x C₁₀ x F_Y x I_{v.tc} x A

where: Q_y is peak flow rate (m³/sec) of average recurrence interval (ARI) of "Y" years C₁₀ is the runoff coefficient (dimensionless) for ARI of 10 years. Rural runoff coefficients are given in Volume 2, figure 5 of Pilgrim (1998), while urban runoff coefficients are given in Volume 1, Book VIII, figure 1.13 of Pilgrim (1998) and construction runoff coefficients are given in Appendix F

Fy is a frequency factor for "Y" years. Rural values are given in Volume 1, Book IV, Table 1.1 of Pilgrim (1998) while urban coefficients are given in Volume 1, Book VIII, Table 1.6 of Pilgrim (1998)

A is the catchment area in hectares (ha) $I_{y,\, tc}$ is the average rainfall intensity (mm/hr) for an ARI of "Y" years and a design duration of "tc" (minutes or hours)

Time of concentration (t_c) = 0.76 x (A/100)^{0.38} hrs (Volume 1, Book IV of Pilgrim, 1998)

Note: For urban catchments the time of concentration should be determined by more precise calculations or reduced by a factor of 50 per cent.

Peak flow calculations, 1

Site	А	tc (mins)	Rainfall intensity, I, mm/hr							
Site	(ha)		1 _{yr,tc}	5 yr,tc	10 yr,tc	20 _{yr,tc}	50 _{yr,tc}	100 yr,tc	C ₁₀	
CAT 1	2.36	11	40.6	60.76	69.36	79.04	91.94	102.2	0.75	

Peak flow calculations, 2

	Frequency		Peak flows							
ARI (yrs)	factor (F _y)	CAT 1 (m³/s)	(m³/s)	(m³/s)	(m³/s)	5 (m³/s)	(m3/s)	Comment		
1 yr.tc	0.8	0.160	(10)	(/0/	(/0/	(/0)	(
5 yr,tc	0.95	0.284								
10 yr,tc	1	0.341								
20 yr,tc	1.05	0.408								
50 yr,tc	1.15	0.520								
100 yr,tc	1.2	0.603								

P2410601JS01V01 RUSLE-Stage 2

SWMP Commentary, Detailed Calculations

3. Volume of Sediment Basins: Type C Soils

Basin volume = settling zone volume + sediment storage volume

Settling Zone Volume

The settling zone volume for Type C soils is calculated to provide capacity to allow the design particle (e.g. 0.02 mm in diameter) to settle in the peak flow expected from the design storm (e.g. 0.25-year ARI). The volume of the basin's settling zone (V) can be determined as a function of the basin's surface area and depth to allow for particles to settle. Peak flowdischarge for the 0.25-year, ARI storm is given by the Rational Formula:

Q tc 0.25 = 0.5 x [0.00278 x C10 x Fy x I 1yr, tc x A] (m3/sec)

Q tc,0.25 = flow rate (m³/sec) for the 0.25 ARI storm event

C₁₀ = runoff coefficient (dimensionless for ARI of 10 years) F. = frequency factor for 1 year ARI storm

I _{1 yr,tc} = average rainfall intensity (mm/hr) for the 1-year ARI storm
A = area of catchment in hectares (ha)

Basin surface area (A) = area factor x Q tc. 0.25 m2

Particle se	ettling velocities unde	er ideal conditions (Se	ection 6.3.5(e))
	Particle Size	Area Factor	
	0.100	170	
	0.050	635	
	0.020	4100	

Volume of settling zone = basin surface area x depth (Section 6.3.5(e)(ii))

Sediment Storage Zone Volume

Sediment Storage Zone volume
In the detailed acluation on Soil Loss Classes 1 to 4 lands, the sediment storage zone can be taken as 100
percent of the settling zone capacity. Alternately designers can design the zone to store the 2-month soil loss as
calculated by the RUSLE (Section 6.3.5(e)(v)). However, on Soil Loss Classes 5, 6 and 7 lands, the zone must
contain the 2-month soil loss as calculated by the RUSLE (Section 6.3.5(e)(v)).

Total Basin Volume

	Q _{tc, 0.25} (m ³ /s)	Area factor	Basin surface area (m²)	Depth of settling zone (m)	Settling zone volume (m³)	Sediment storage volume (m³)	Total basin volume (m³)	Basin shape			
Site								L:W Ratio	Length (m)	Width (m)	
CAT 1	0.080	4100	328	0.6	197	197	393	3	31.4	10.5	

P2410601JS01V01 RUSLE-Stage 2

- SEDIMENT BASIN IS NOT REQUIRED FOR STAGE 1 AS TOTAL SOIL LOSS IS LESS THAN 150m³/yr (147m³/ha/yr x 0.54ha = 79.38m³/yr < 150m³/yr).
- SEDIMENT BASIN IS REQUIRED FOR STAGE 2, AS TOTAL SOIL LOSS IS GREATER THAN 150m³/yr (135m³/ha/yr x 2.4ha = 324m³/yr > 150m³/yr).

	REV	DESCRIPTION	DATE	DRAWN	DESIGNED	CHECKED	APPRVD	SCALE
z	C	MINOR AMENDMENTS	20/03/2025	SSH	RK	AVG	GT	
ASS,	В	MINOR AMENDMENTS	11/03/2025	SSH	RK	AVG	GT	
USER: NHASSAN	Α	INITIAL RELEASE	25/02/2025	SSH	RL/PC	AVG	GT	
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PROJECT MANAGER CLIENT

DATUM

BLUE SOX DEVELOPMENTS PROJECT NAME/PLANSET TITLE

PROPOSED 20 LOT SUBDIVISION CONCEPT CIVIL ENGINEERING PLANS 3 MEMORY AVE CROOKWELL NSW 2583



Consulting Engineers Environment

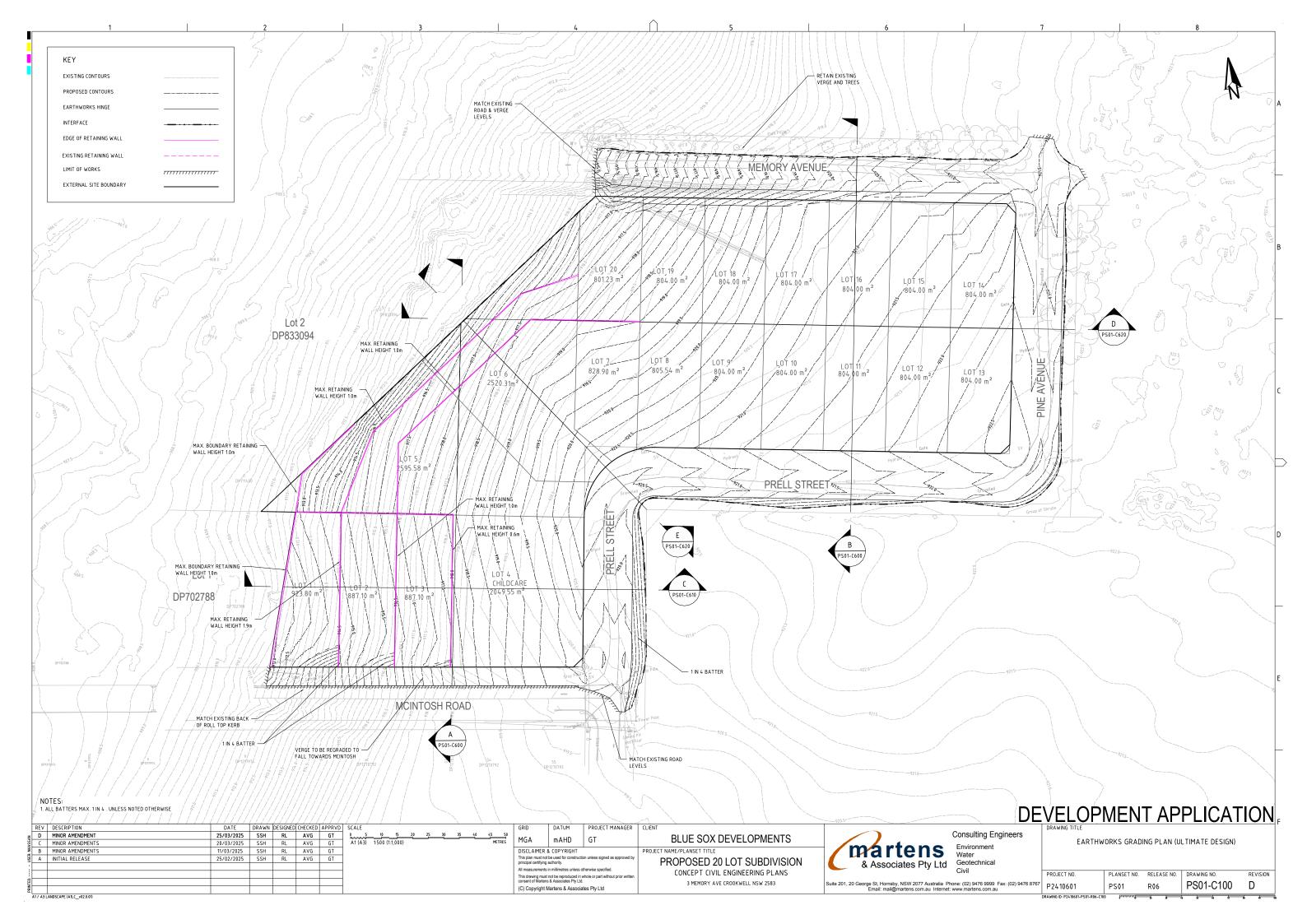
Suite 201, 20 George St, Hornsby, NSW 2077 Australia Phone: (02) 9476 9999 Fax: (02) 9476 8767 Email: mail@martens.com.au Internet: www.martens.com.au

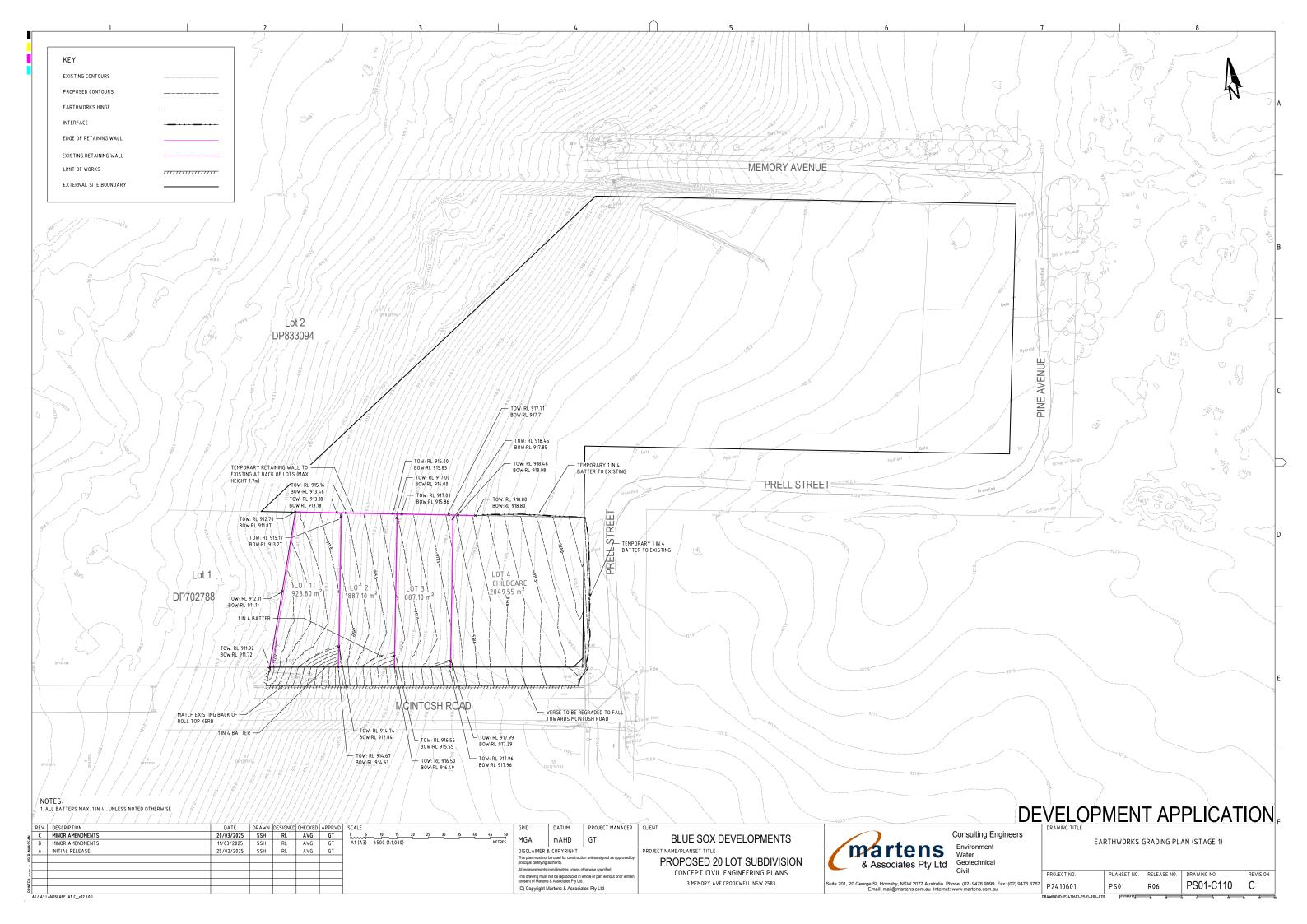
DEVELOPMENT APPLICATION

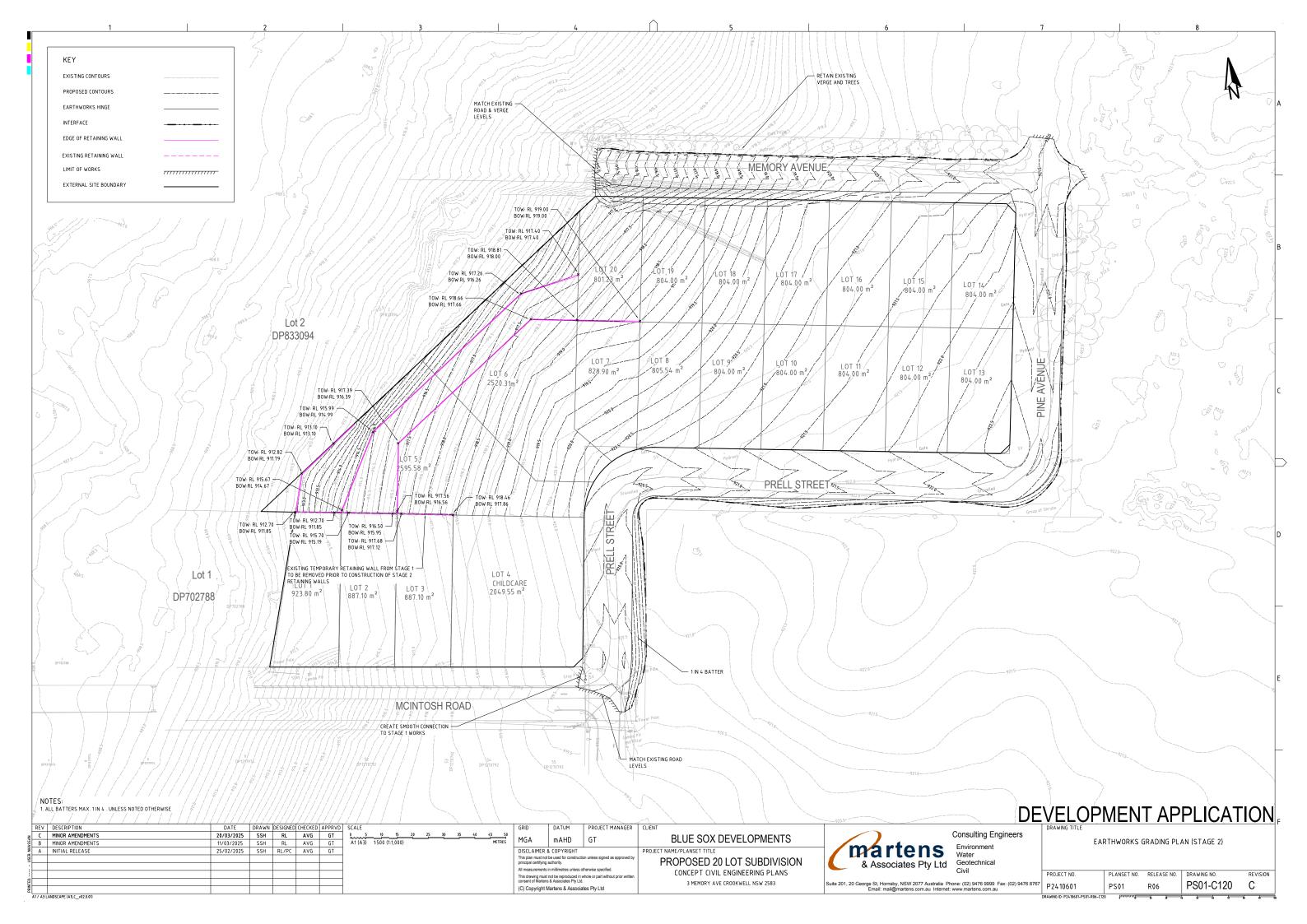
SOIL AND WATER MANAGEMENT DETAILS RUSLE CALCULATIONS

PRO IFCT NO PLANSET NO. RELEASE NO. DRAWING NO. REVISION PS01-B340 С PS01 R06

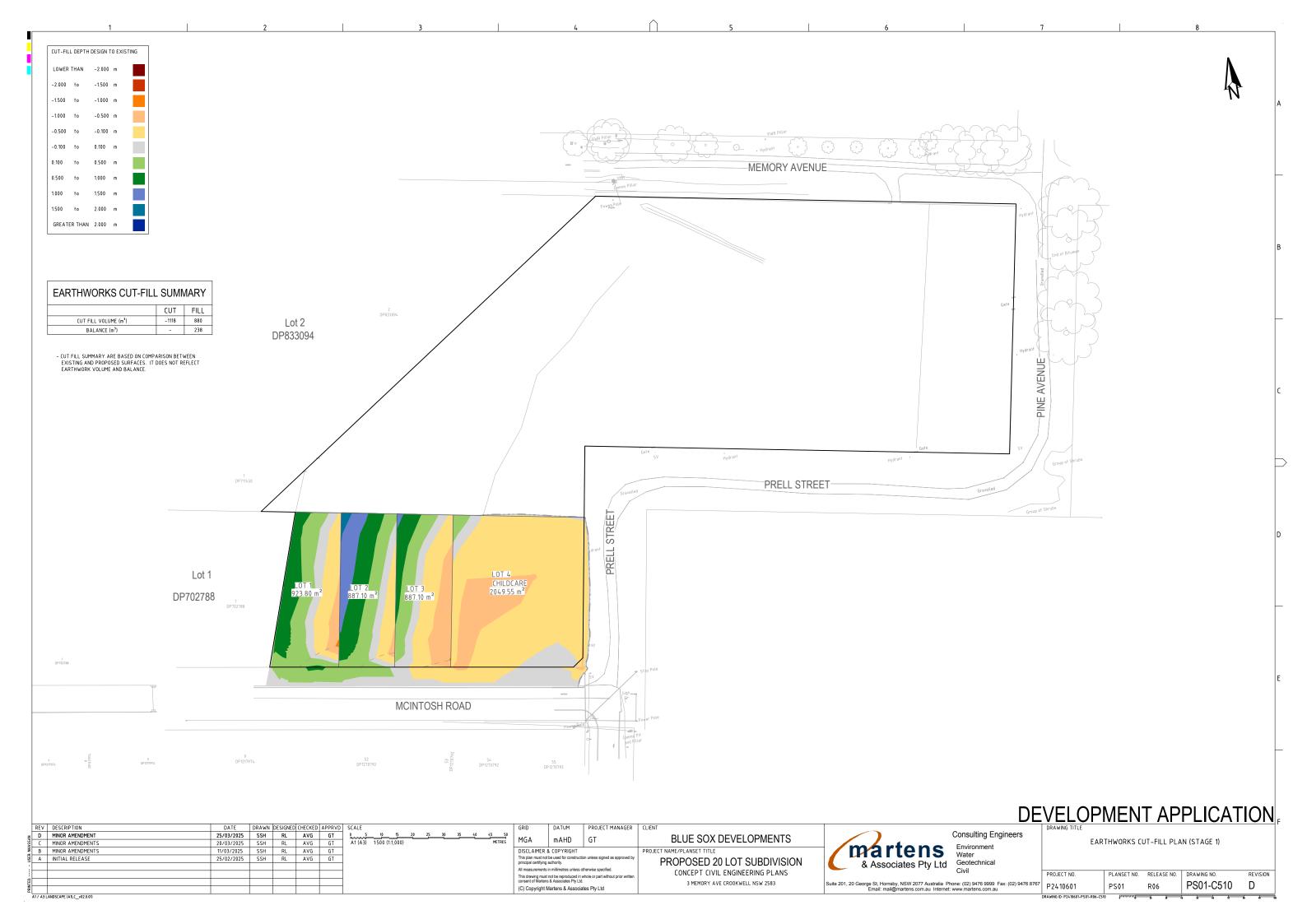
A1 / A3 LANDSCAPE (A1LC_v02.0.01)



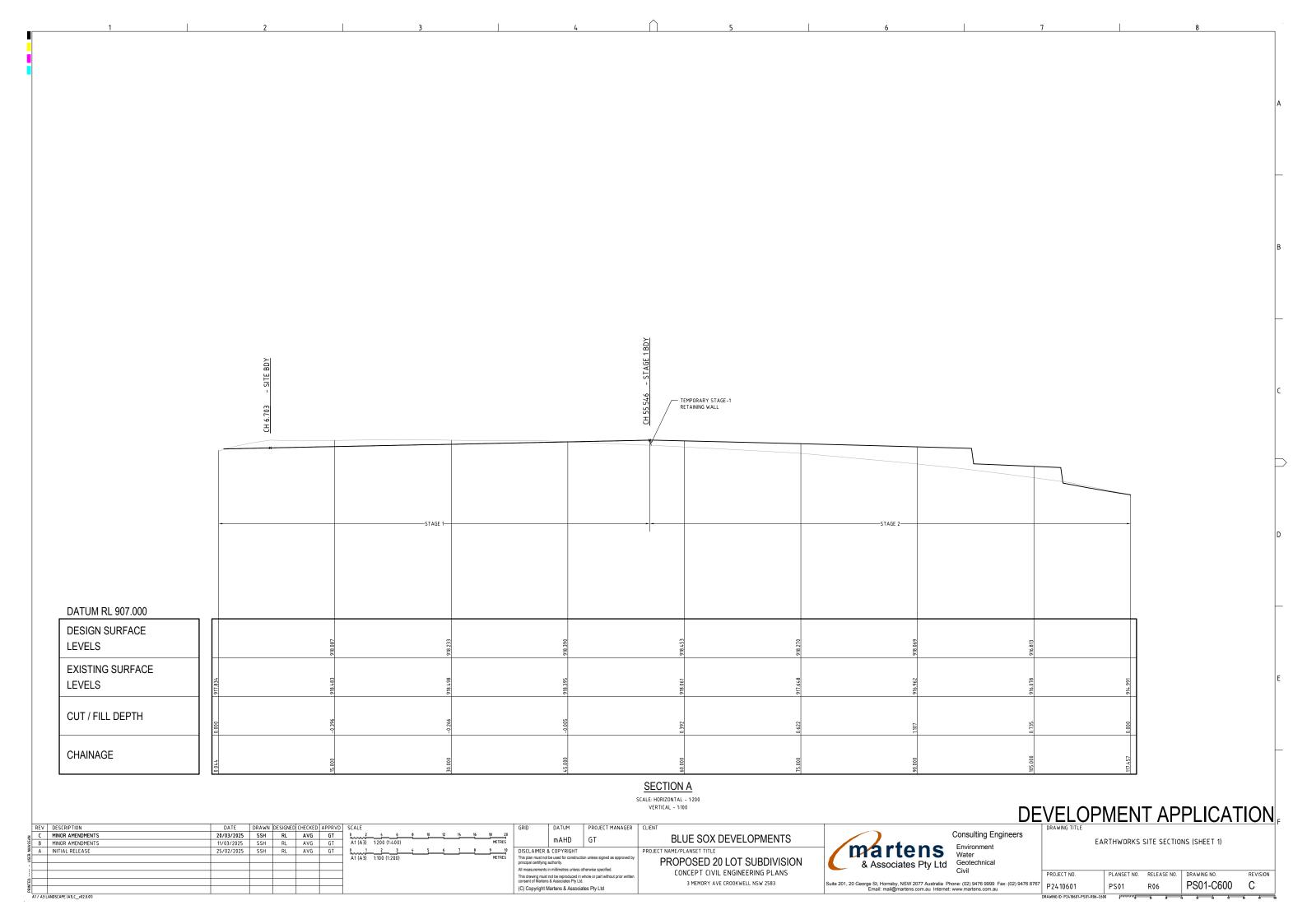


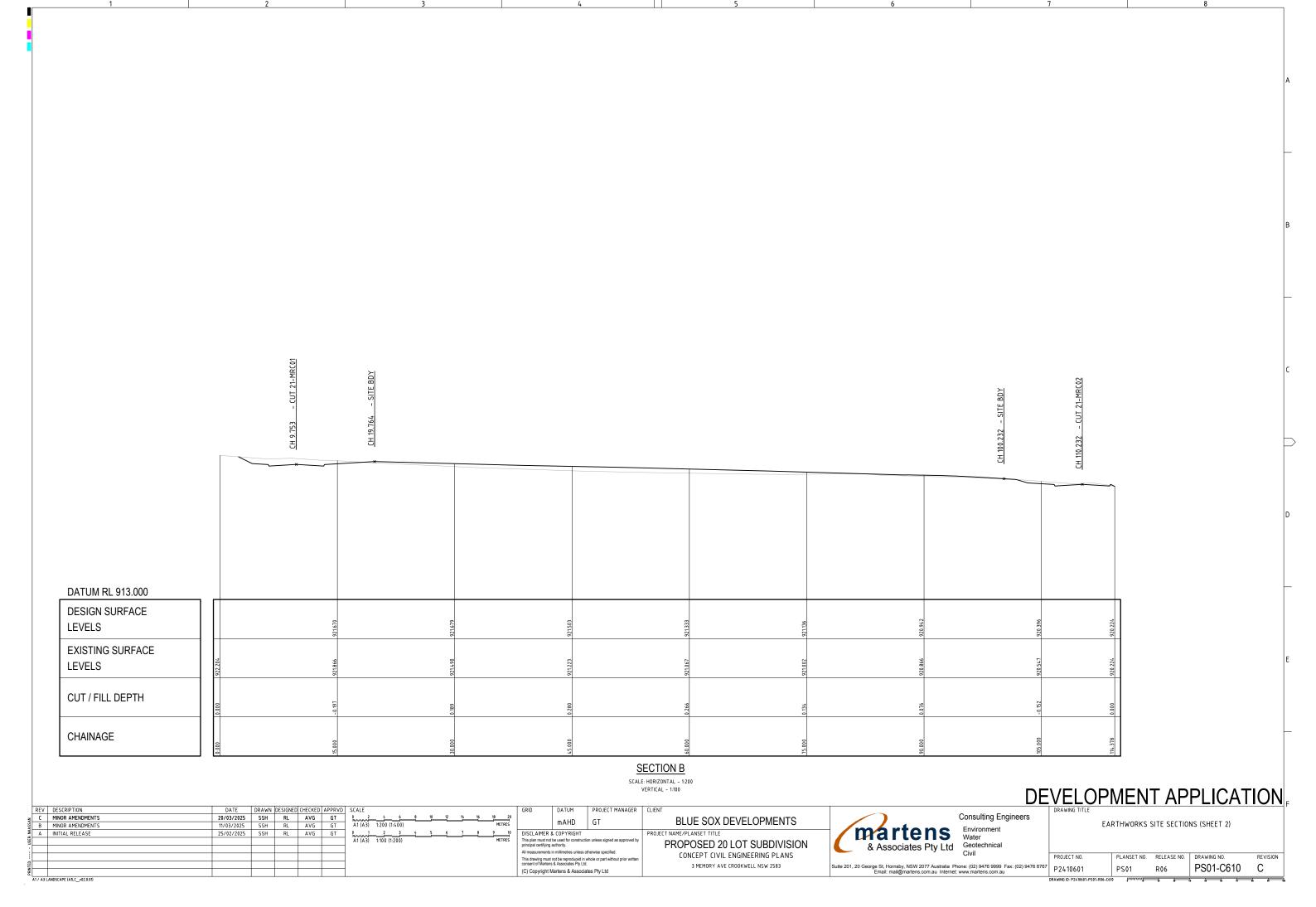


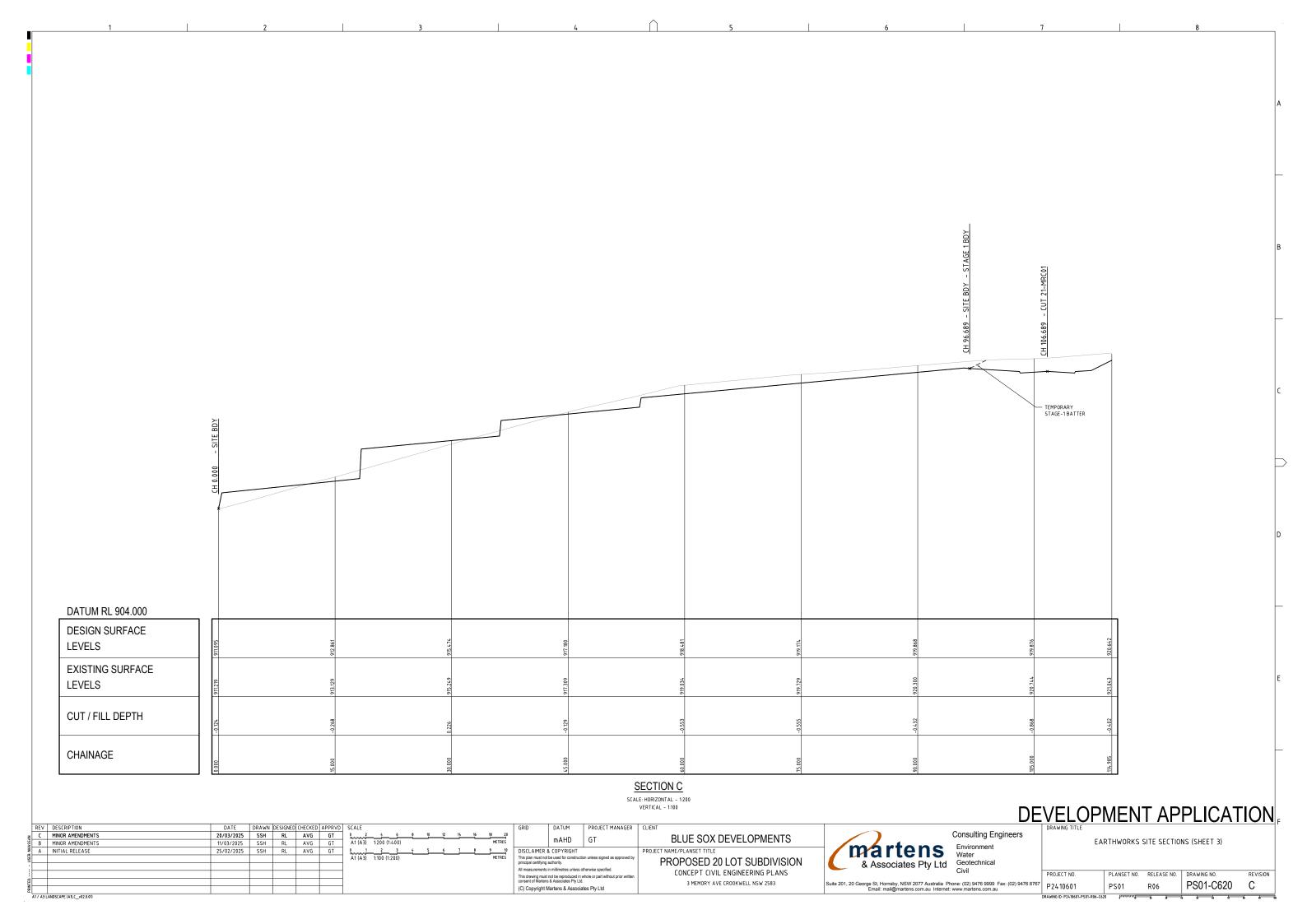


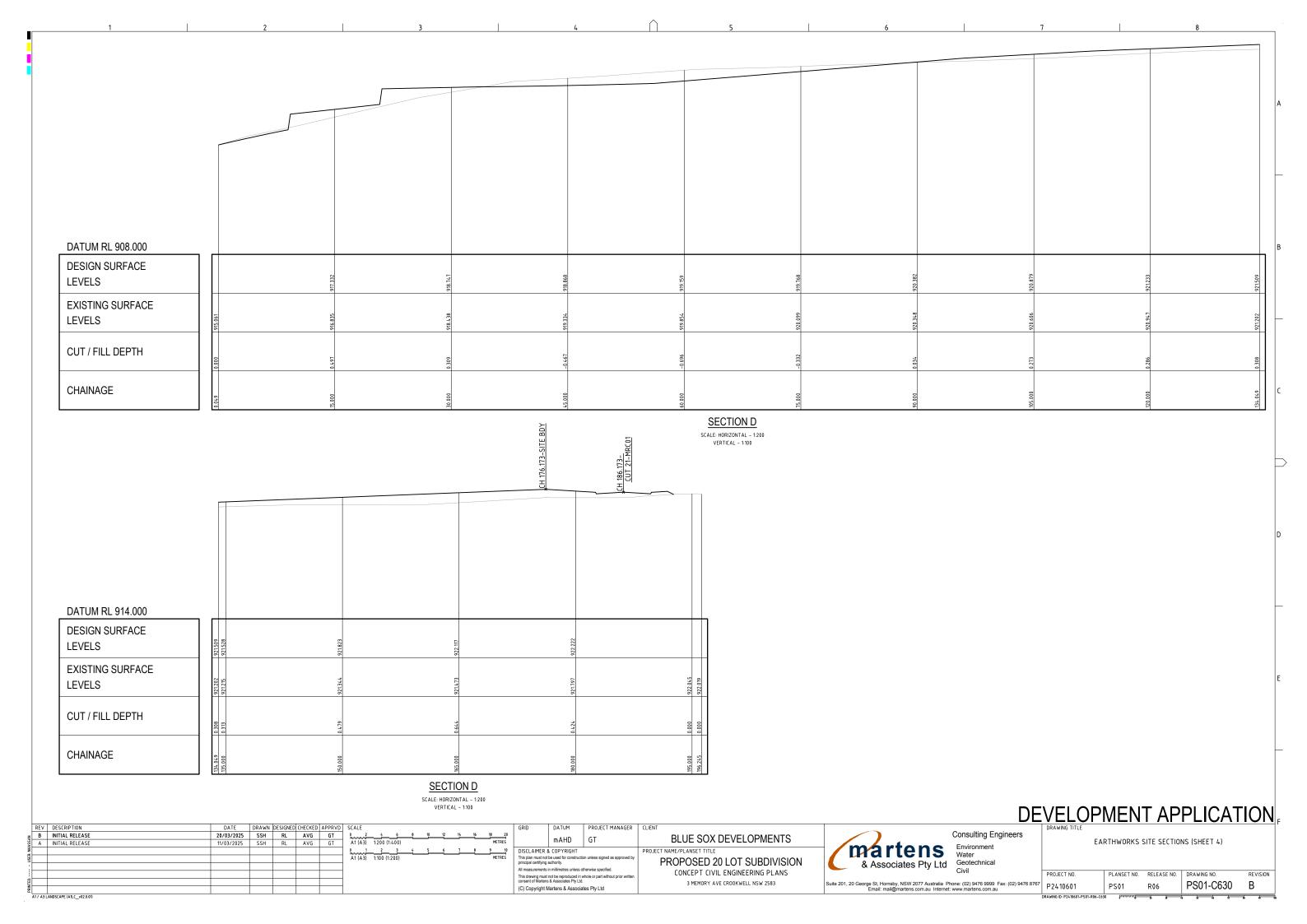


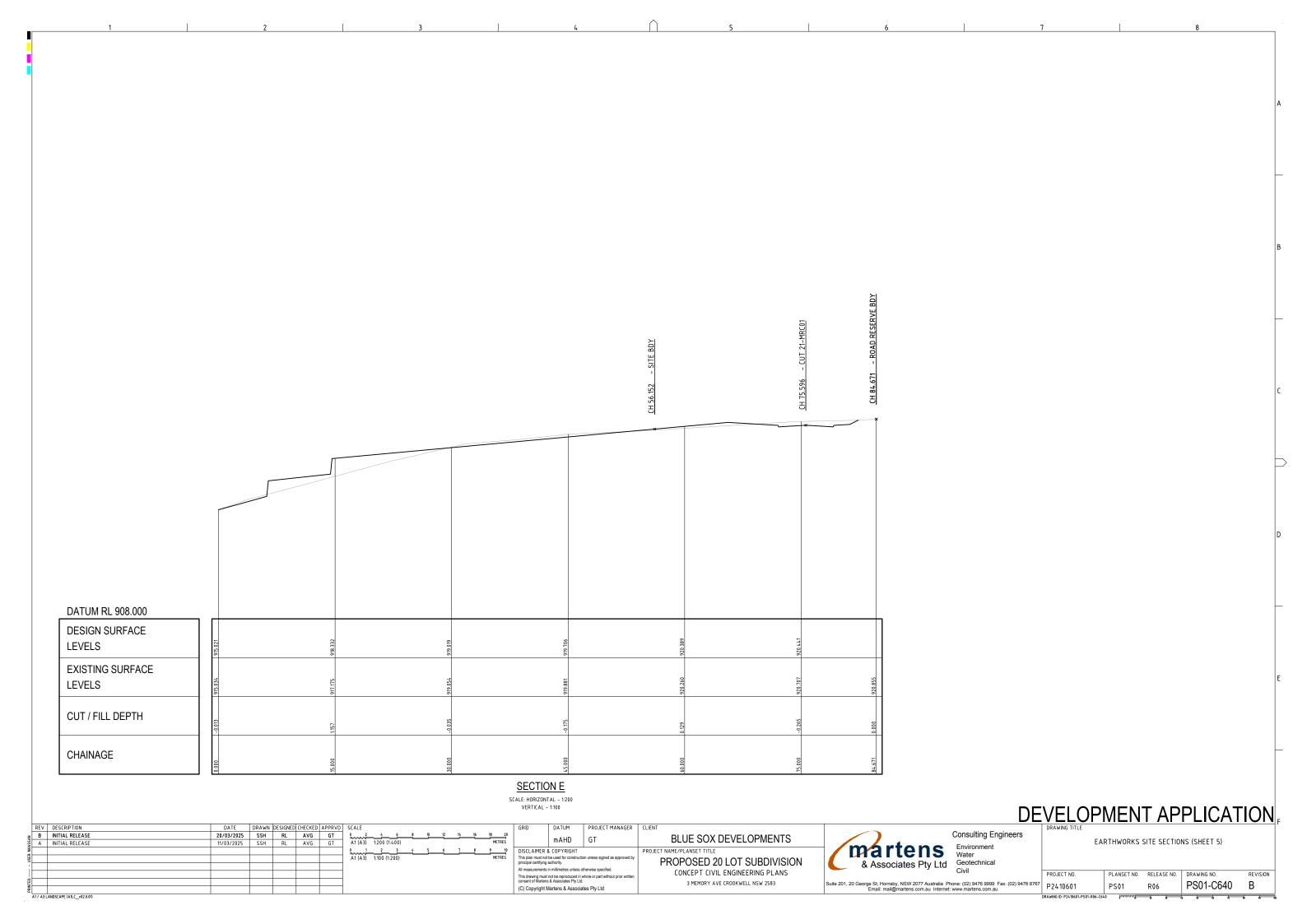


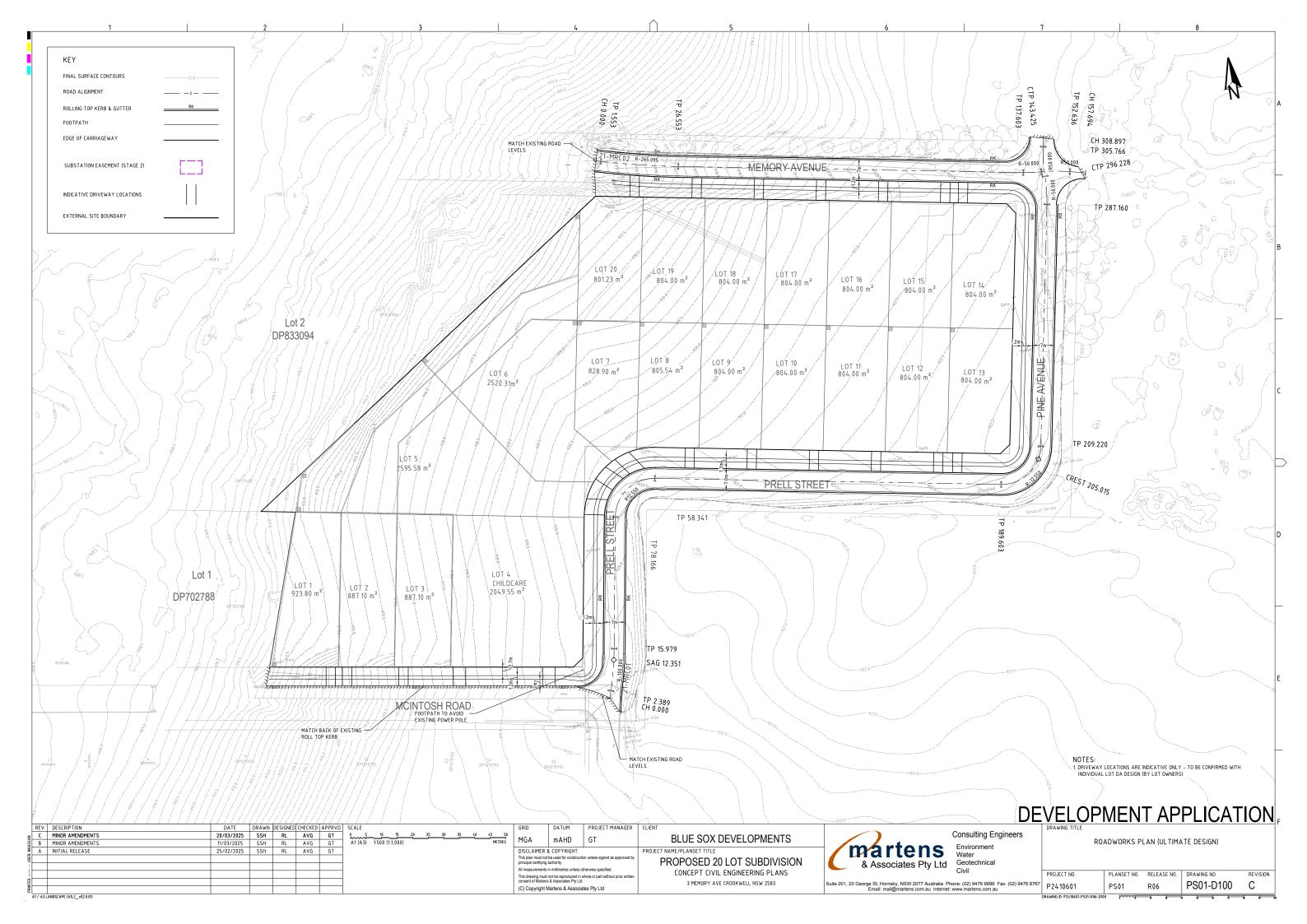


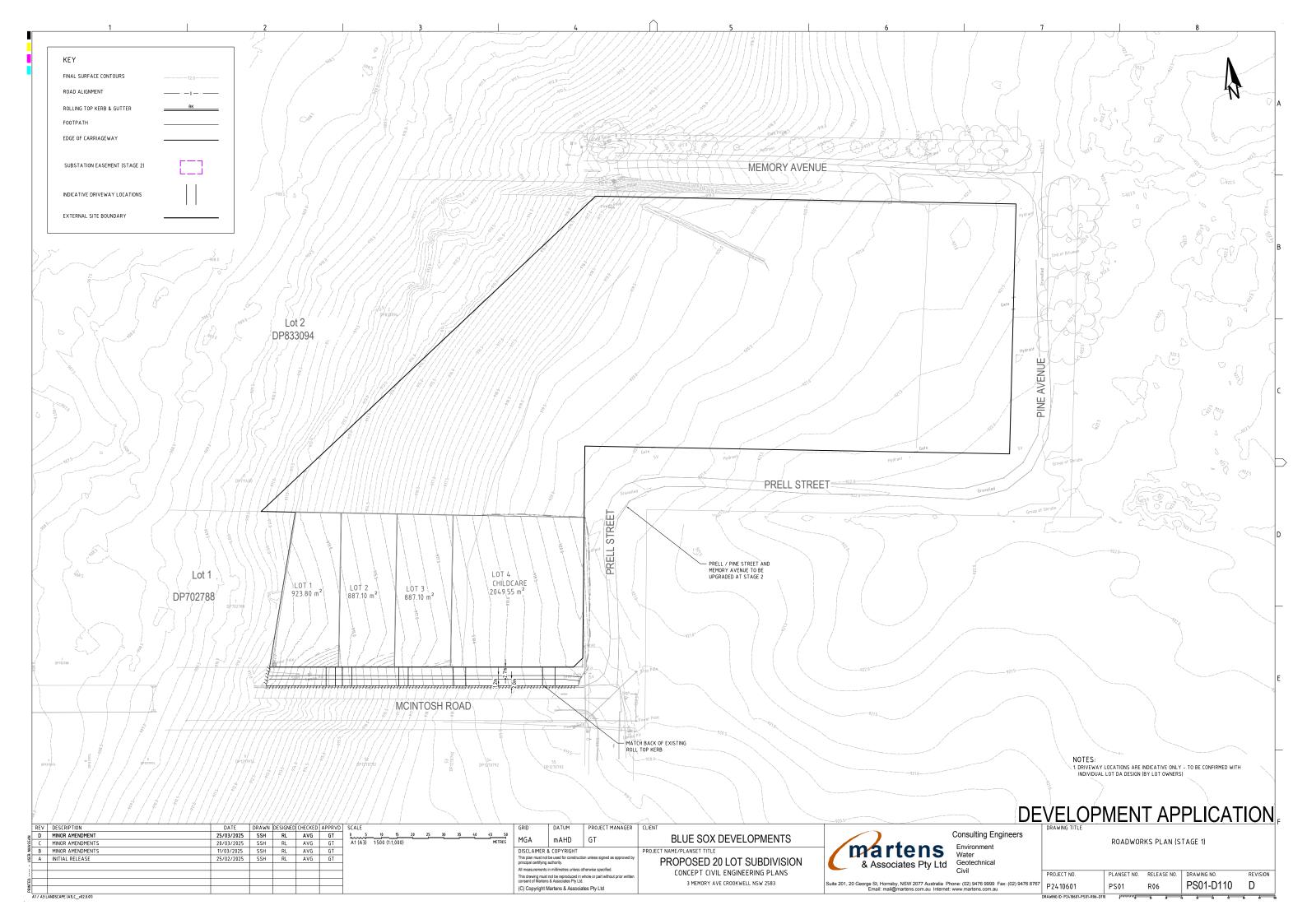


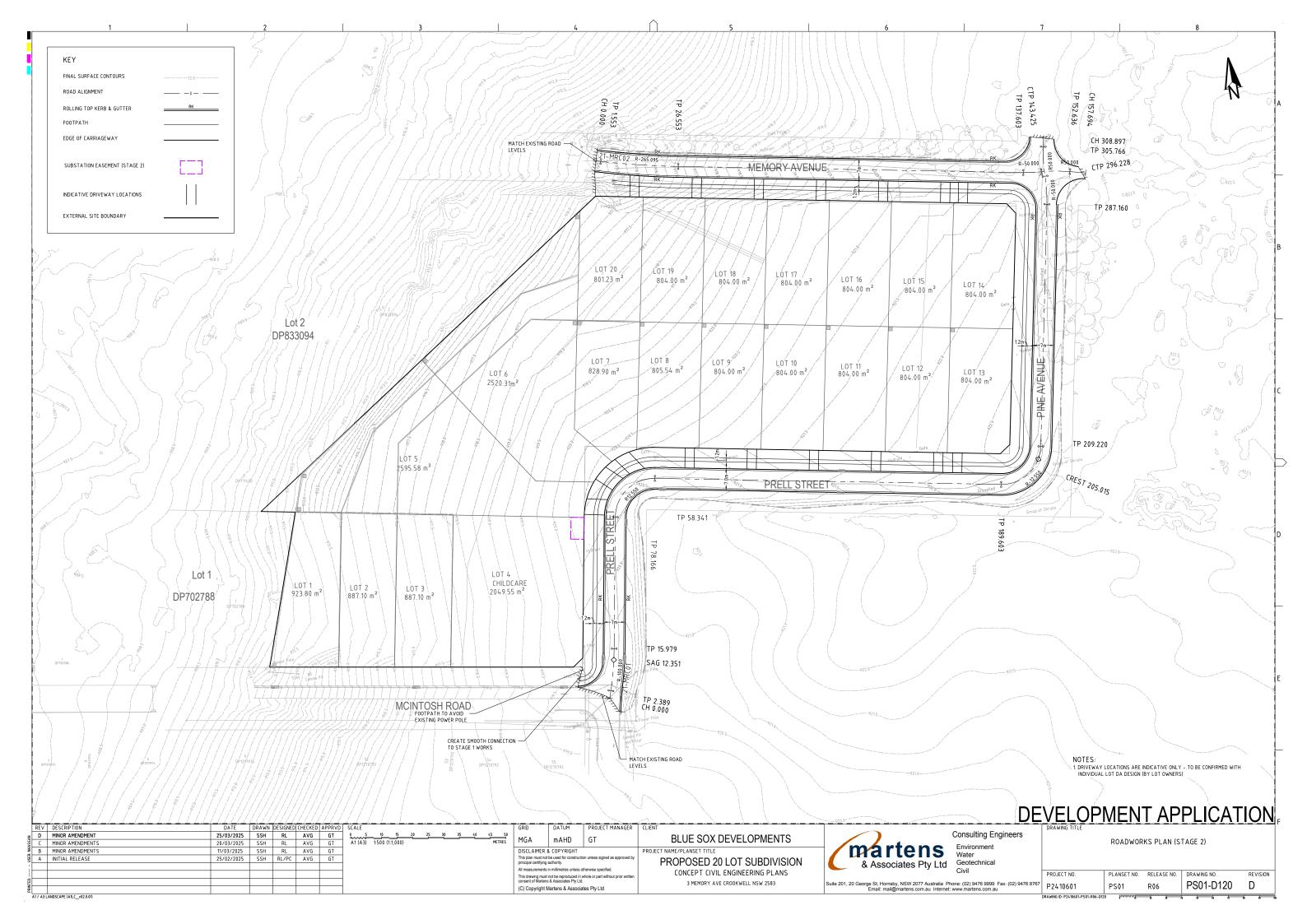


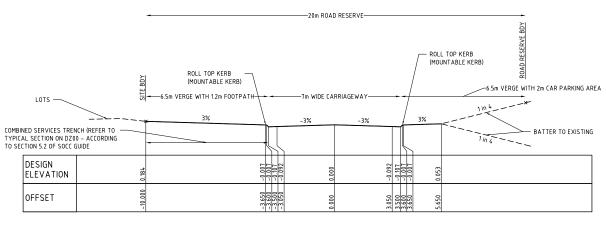












PRELL STREET (21-MRC01) TYPICAL SECTION (CH: 0.00 TO 287.00) SCALE 1:100

VERTICAL CURVE LENGTH (m) VERTICAL CURVE RADIUS (m) VERTICAL GRADE (%) VERTICAL GRADE (1 IN ...) HORIZONTAL CURVE RADIUS (m) DATUM RL 912.000 DESIGN SURFACE 920.300 920.327 922.515 LEVELS **EXISTING SURFACE** 920.747 922.228 **LEVELS** CUT / FILL DEPTH 0.336 0.308 0.286 0.286 CHAINAGE 209.220

PRELL STREET / PINE AVENUE (21-MRC01) LONG. SECTION

SCALE: HORIZONTAL - 1:500

VERTICAL - 1:100

						DE	VELOPI	MENT AF	PPLICAT	TION
REV	DESCRIPTION	DATE DRAWN DESIGNED CHECKED APPRVD	SCALE	GRID DATUM PROJECT MANAGER	CLIENT		DRAWING TITLE			
₹ (MINOR AMENDMENTS	20/03/2025 SSH RL AVG GT	0 5 10 15 20 25 30 35 40 45 50	mAHD GT	BLUE SOX DEVELOPMENTS	Consulting Engineers				
ASS, B	MINOR AMENDMENTS	11/03/2025 SSH RL AVG GT	A1 (A3) 1:500 (1:1,000) METRES	IIIAND UT	DEUL SON DEVELOP MENTS	Environment) 1 (21–MRC01) LONGITI		
ĕ A	INITIAL RELEASE	25/02/2025 SSH RL AVG GT	0 1 2 3 4 5 6 7 8 9 10	DISCLAIMER & COPYRIGHT	PROJECT NAME/PLANSET TITLE	Martiens Water	/	AND TYPICAL SECTION	IS (SHEET 1)	
USE			A1 (A3) 1:100 (1:200) METRES	This plan must not be used for construction unless signed as approved by principal certifying authority.	PROPOSED 20 LOT SUBDIVISION	O A a a a distance Distance Geotechnical				
- :				All measurements in millimetres unless otherwise specified		& Associates Pty Ltd Geotechnical				
				This drawing must not be reproduced in whole or part without prior written	CONCEPT CIVIL ENGINEERING PLANS	CIVII	PROJECT NO.	PLANSET NO. RELEASE NO	. DRAWING NO.	REVISION
£				consent of Martens & Associates Pty Ltd.	3 MEMORY AVE CROOKWELL NSW 2583	Suite 201, 20 George St, Hornsby, NSW 2077 Australia Phone: (02) 9476 9999 Fax: (02) 9476 8767	P2410601	PS01 R06	PS01-D200	\sim
Ē.	NOCCOLO LANCE DO A AND			(C) Copyright Martens & Associates Pty Ltd		Email: mail@martens.com.au Internet: www.martens.com.au	P Z 4 10 6 0 1		F 30 1-D200	

-20m ROAD RESERVE-- ROLL TOP KERB (MOUNTABLE KERB) ROLL TOP KERB (MOUNTABLE KERB) MIN 6.5m VERGE WITH FOOTPATH -7m WIDE CARRIAGEWAY-RETAIN EXISTING -VERGE 1 in 6.67 - - -- CUT EXISTING BATTER TO ACHIEVE SITE ACCESS COMBINED SERVICES TRENCH (REFER TO TYPICAL SECTION ON DZ00-ACCORDING TO SECTION 5.2 OF SOCC GUIDE) -0.092 -0.107 -0.007 ELEVATION OFFSET MEMORY AVENUE (21-MRC02) TYPICAL SECTION SCALE 1:100 VERTICAL CURVE LENGTH (m) VERTICAL CURVE RADIUS (m) VERTICAL GRADE (%) VERTICAL GRADE (1 IN ...) HORIZONTAL CURVE RADIUS (m) DATUM RL 907.000 DESIGN SURFACE **LEVELS EXISTING SURFACE** LEVELS CUT / FILL DEPTH 0.091 CHAINAGE MEMORY AVENUE (21-MRC02) LONG. SECTION SCALE: HORIZONTAL - 1:500 VERTICAL - 1:100

REV DESCRIPTION
C MINOR AMENDMENTS
B MINOR AMENDMENTS
 DATE
 DRAWN
 DESIGNED CHECKED APPRVD
 SCALE

 20/03/2025
 SSH
 RL
 AVG
 GT

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 SSH
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 A1 (A3
 0 5 10 15 A1 (A3) 1:500 (1:1,000) 0 1 2 3 A1 (A3) 1:100 (1:200) A INITIAL RELEASE 25/02/2025 SSH RL AVG GT

DATUM PROJECT MANAGER CLIENT mAHD GT DISCLAIMER & COPYRIGHT

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PROJECT NAME/PLANSET TITLE PROPOSED 20 LOT SUBDIVISION CONCEPT CIVIL ENGINEERING PLANS 3 MEMORY AVE CROOKWELL NSW 2583

BLUE SOX DEVELOPMENTS



Consulting Engineers

Suite 201, 20 George St, Hornsby, NSW 2077 Australia Phone: (02) 9476 9999 Fax: (02) 9476 8767 Email: mail@martens.com.au Internet: www.martens.com.au

DE	EVELOPMENT APPLICATION

	PROJECT NO.	PLANSET NO.	RELEASE NO.	DRAWING NO.	REVISION
37	P2410601	PS01	R06	PS01-D210	С
	DRAWING ID: P2410601-PS01-R06-D21	0 հոոոո ւ՞		9 6 A	ú á

ROAD 1 (21-MRC01) LONGITUDINAL SECTION

AND TYPICAL SECTIONS (SHEET 2)

E ______| A1 / A3 LANDSCAPE (A1LC__v02.0.01)

TYPICAL UTILITY PROVIDER ALLOCATION

REF. NSW GUIDE TO CODES AND PRACTICES FOR STREETS OPENING SECTION 5.

NOTES:

- 1. IF INSTALLING ASSETS IN REGIONAL AREAS CONTACT THE LOCAL COUNCIL FOR COUNCIL SPECIFIC REQUIREMENTS.

 2. WHERE A UTILITY/SERVICE PROVIDER PROVIDING UNDERGROUND SERVICES WISHES TO ENCROACH ON SPACE ALLOCATED TO ANOTHER UTILITY/SERVICE PROVIDER, IT SHOULD CONSULT AND SEEK AGREEMENT WITH THE OTHER. BOTH UTILITY/SERVICE PROVIDERS SHOULD RECORD SUCH ENCROACHMENTS ON THEIR RESPECTIVE MAPPING SYSTEMS.

 3. THE NARROWER WATER ALLOCATION SHOWN MAY NOT BE SUFFICIENT TO INCLUDE RECYCLED WATER MAINS.

 4. THE PREFERED POSITION FOR POLES OR STREET LIGHTING COLUMNS IS WITHIN 300MM OF THE PROPERTY ALIGNMENT. SOME ALTERNATIVE POSITIONS ARE SHOWN BUT LOCATION SHOULD BE CONSISTENT WITH THE OVERALL COST TO THE PUBLIC WHILE CONSIDERING SAFETY REQUIREMENTS.

 5. WHERE THE ERECTION OF POWER POLES IN THE O-1200MM ALLOCATION IS IMPRACTICABLE, THESE MAY BE LOCATED IN THE WATER ALLOCATION BY AGREEMENT WITH THE APPROPRIATE PUBLIC AUTHORITY.

 6. NO SPECIFIC ALLOCATION FOR TREES CAN BE IDENTIFIED FOR FOOTWAYS UP TO 3600MM WIDE. CONSULTATION WITH UTILITY/SERVICE PROVIDERS IS REQUIRED AND DUE REGARD MUST BE GIVEN TO TREE SPECIES AS OUTLINED IN 6.5 STREET TREES.

 7. PILLARS/PEDESTALS/SERVICE PITS ETC. SHOULD BE LOCATED IN A POSITION THAT IS SET BACK FROM STREET INTERSECTIONS.

 8. SEE SECTION 6.6 FOR GUIDANCE ON NEW POLES AND POLE REPLACEMENTS.

 9. SEWER PRESSURE MAINS TO BE LADIN IN WATER ALLOCATION DEPTRIT THAN WATER MAINS.

 10. VACUUM SEWERS TYPICALLY LAID IN PROPERTY BUT COULD ALSO BE IN WATER ALLOCATION.

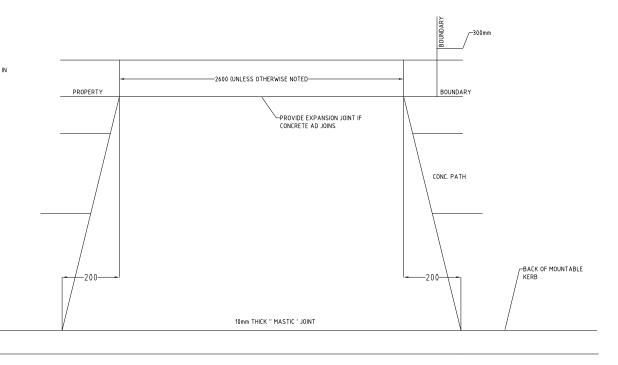
 11. FOR STRUCTURES TO BE ERECTED OVER THE ELECTRICAL DISTRIBUTORS FOOTPATH ALLOCATION FOR THE ELECTRICAL DISTRIBUTOR MUST BE CONSULTED IN ACCORDANCE WITH CLAUSE 3.3 CUSTOMERS STRUCTURE SERVICE AND INSTALLATION ROLLES OF NEW SOUTH WALES.

 12. IF BOTH FOOTPATHS ARE ABLE TO BE USED, THE ASSETS SHOULD BE ABLE TO BE BETTER DISTRIBUTED ACROSS BOTH SIDES OF THE RODAWAY SO THAT THERE IS SPACE TO INSTALL INFRASTRUCTURE AT THE MOST APPROPRIATE LOCATION WITH RESPECT TO MINIMIZING OVERALL COST TO THE CUSTOMERS.

R10-170 600

TYPICAL MOUNTABLE / TOLL TOP KERB

NOT TO SCALE



TYPICAL DRIVEWAY DETAIL

NOTES:

- IN LES:

 1. ALL EXPOSED CONCRETE EDGES SHALL BE ROUNDED TO 10mm RADIUS

 2. PROVIDE 30mm TOP COVER TO REINFORCING FABRIC.

 3. CONCRETE SHALL BE A MINIMUM OF 25MPa

 4. MINIMUM CONCRETE SLAB THICKNESS SHALL BE: RESIDENTIAL 125mm, INDUSTRIAL AND COMMERCIAL 150mm

 5. EXPANSION JOINTS "E.J." SHALL BE FILLED WITH 10mm THICK BITUMEN IMPREGNATED PREFORMED JOINTING

 MATERIAL AT FOOTPATH EDGE.
- MATEMAL AT FOUTPATH EDUE.
 6. FINISHED SUPFACE TEXTURE SHALL BE A STEEL FLOAT FINISH FOR VEHICULAR GUTTER CROSSINGS, BROOMED FINISH FOR VEHICULAR FOOTPATH CROSSINGS AND THE CONCRETE IS TO BE COATED WITH A SUITABLE CURING

DATUM

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TO DRIVEWAYS TO BE CONSTRUCTED AT RIGHT ANGLE TO KERB LINE.

8. ALL DRIVEWAYS TO COMPLY WITH UPPER LACHLAN SHIRE COUNCIL SPECIFICATIONS.

DEVELOPMENT APPLICATION

	REV	DESCRIPTION	DATE	DRAWN	DESIGNED	CHECKED	APPRVD	SCALE
×	С	MINOR AMENDMENTS	20/03/2025	SSH	RL	AVG	GT	
ASS,	В	MINOR AMENDMENTS	11/03/2025	SSH	RL	AVG	GT	
₹	Α	INITIAL RELEASE	25/02/2025	SSH	RL	AVG	GT	
USER								
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PROJECT MANAGER CLIENT

PROJECT NAME/PLANSET TITL PROPOSED 20 LOT SUBDIVISION CONCEPT CIVIL ENGINEERING PLANS

BLUE SOX DEVELOPMENTS

3 MEMORY AVE CROOKWELL NSW 2583



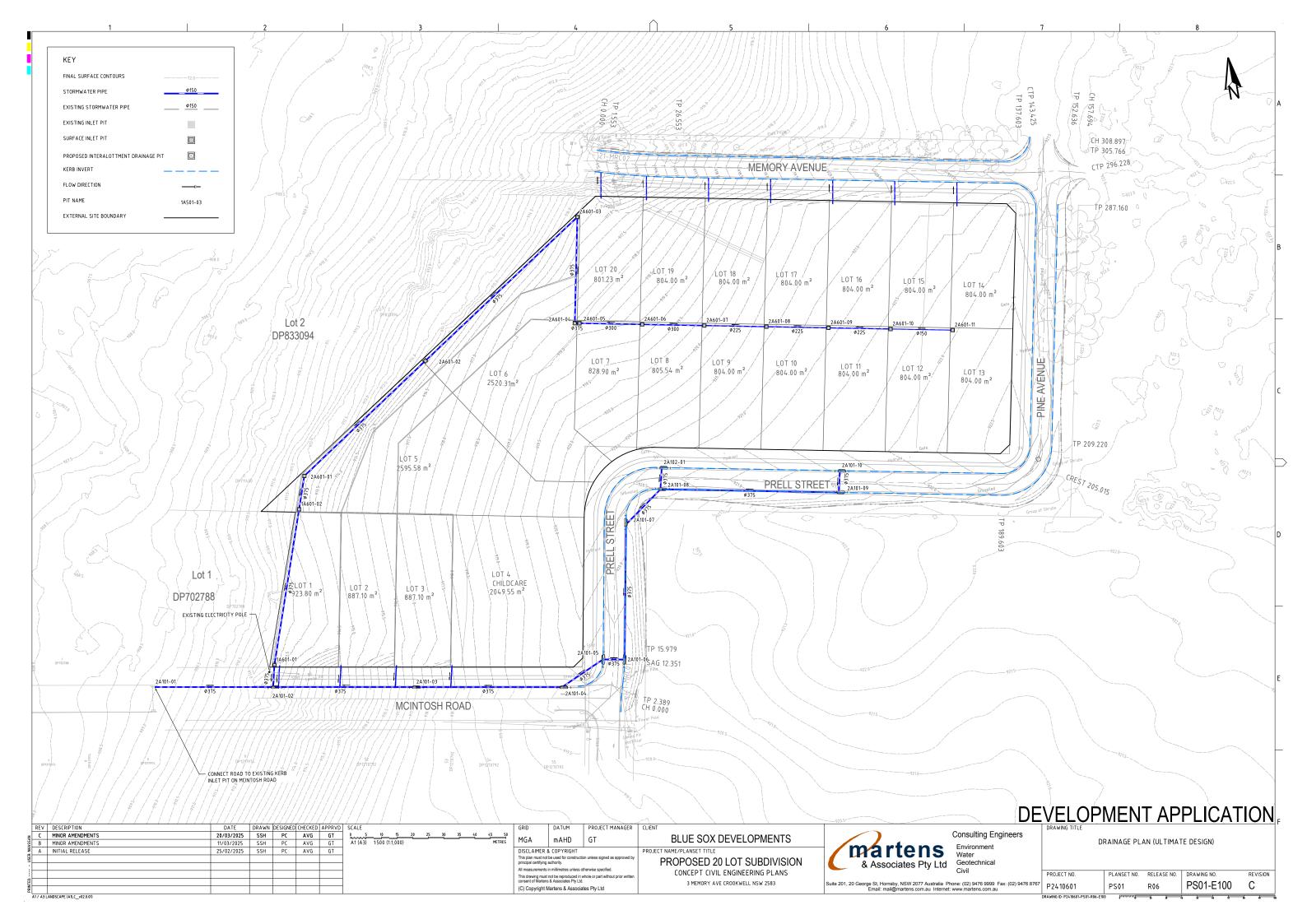
Consulting Engineers Water

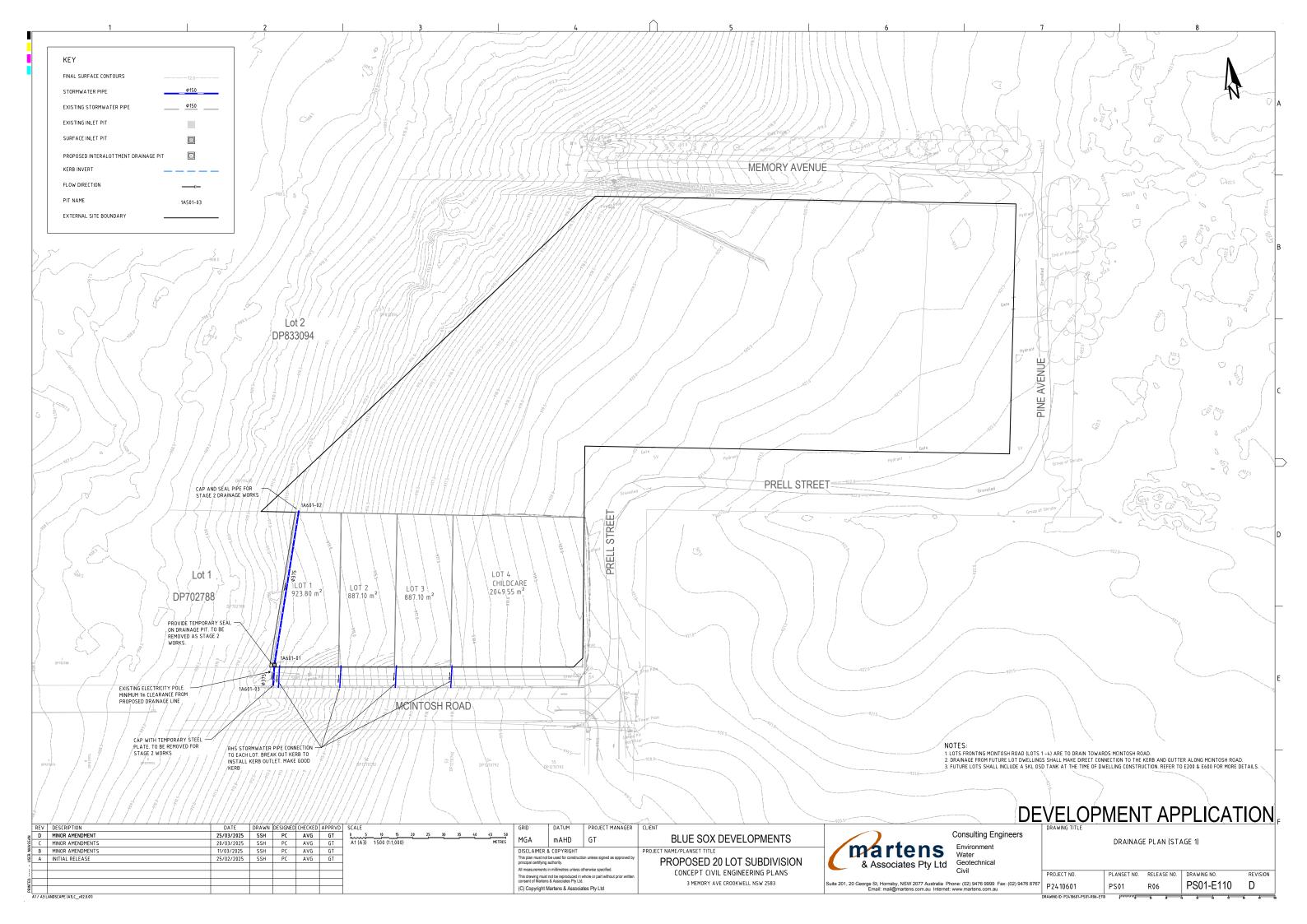
uite 201, 20 George St, Hornsby, NSW 2077 Australia Phone: (02) 9476 9999 Fax: (02) 9476 8767 Email: mail@martens.com.au Internet: www.martens.com.au

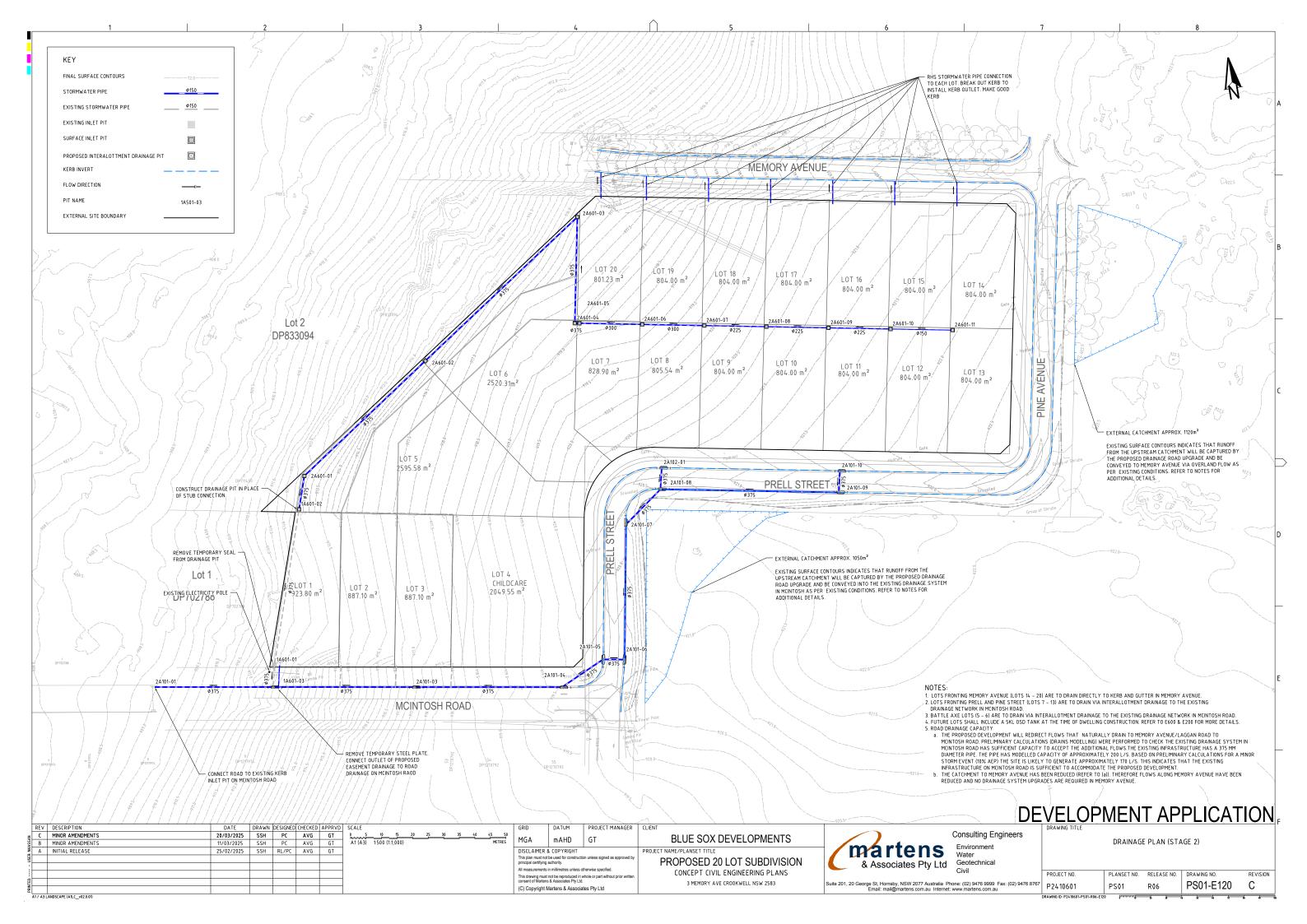
NG TITLE	
	COMBINED SERVICES TRENCH
	AND DRIVEWAY GUIDELINES DETAILS

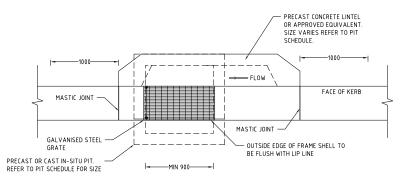
PLANSET NO. RELEASE NO. DRAWING NO REVISION PS01-DZ00 С P2410601 PS01

A1 / A3 LANDSCAPE (A1LC_v02.0.01)



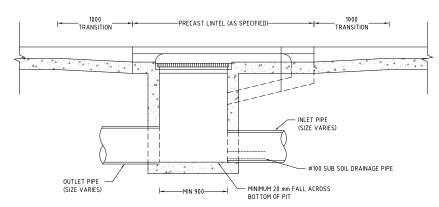




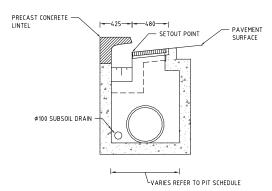


TYPICAL KERB INLET PIT

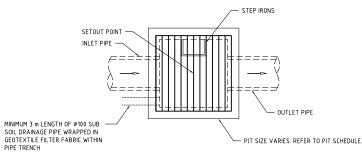
SCALE: 1:25



TYPICAL KERB INLET PIT LONGITUDINAL SECTION



TYPICAL KERB INLET PIT CROSS SECTION



TYPICAL PIT SCALE: 1:25

- PIT DETAILS:

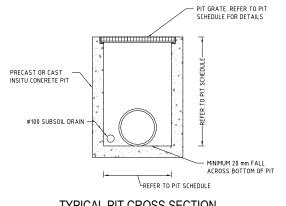
 1. PROVIDE STEP IRONS WHERE PIT IS DEEPER THAN 1.0m. STEP IRONS SHALL BE LOCATED ON A WALL OF THE PIT THAT IS ACCESSIBLE AND SAFE
 TO CLIMB FOR THE FULL HEIGHT OF THE PIT.

 2. 100mm DIA CORRUGATED SUBSOIL PIPE, 3m LONG AND WRAPPED IN FILTER FABRIC, SHALL BE PROVIDED TO THE UPSTREAM PIT WALL.

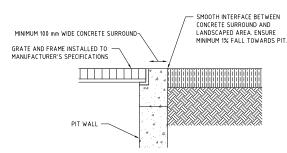
 3. LOCATE LINTELS CENTRALLY IN SAG POINTS.

 4. APPROVED PRECAST CONCRETE PITS IPART OR TOTALI MAY BE USED IN ACCORDANCE WITH THE MANUFACTURERS INSTRUCTIONS.

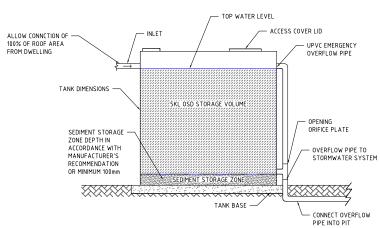
 5. GRATE SHALL BE WELDLOCK HINGED HOT DIPPED GALVANISED. SET PARALLEL TO GUTTER AND WITH LOCKING CLIP.



TYPICAL PIT CROSS SECTION SCALE 1:25



TYPICAL INTERFACE - PIT IN SOFT LANDSCAPING



TYPICAL ABOVE GROUND OSD TANK

DEVELOPMENT APPLICATION

REV DESCRIPTION 20/03/2025 SSH PC AVG GT
11/03/2025 SSH PC AVG GT Consulting Engineers MINOR AMENDMENTS **BLUE SOX DEVELOPMENTS** GT DRAINAGE DETAILS MINOR AMENDMENTS martens DISCLAIMER & COPYRIGHT PROJECT NAME/PLANSET TITLE INITIAL RELEASE 25/02/2025 AVG GT Water PROPOSED 20 LOT SUBDIVISION & Associates Pty Ltd All measurements in millimetres unless otherwise specified CONCEPT CIVIL ENGINEERING PLANS PLANSET NO. RELEASE NO. DRAWING NO. REVISION This drawing must not be reproduced in whole or part without prior writte consent of Martens & Associates Pty Ltd. Suite 201, 20 George St, Hornsby, NSW 2077 Australia Phone: (02) 9476 9999 Fax: (02) 9476 8767 Email: mail@martens.com.au Internet: www.martens.com.au 3 MEMORY AVE CROOKWELL NSW 2583 PS01-E200 С P2410601 PS01 (C) Copyright Martens & Associates Pty Ltd A1 / A3 LANDSCAPE (A1LC_v02.0.01)

PROJECT MANAGER | CLIENT

E T27524.236 (0-1099) B F727524.236 (0-1099) B F727525.717 (0-1099) B F727525.717 (1A601-02) PIPE SIZE & CLASS 0375 Des RCP2 1.1% Des uPVC 1.3% Datum R.L DEPTH TO INVERT PROPOSED LEVEL EXISTING LEVEL CHAINAGE

LINE 1A601	(STAGE 1)
------------	-----------

INVERT LEVEL

				PIT SCH	IEDULI	E (STAG	E 1)					
PIT				INTERNAL		INLET		OUTLET		PIT		
NAME	TYPE	EASTING	NORTHING	WD	LEN	DIA	INV LEV	DIA	INV LEV	SETOUT RL	DEPTH	REMARKS
1A601-02	STUB CONNECTION	727541.635	6185320.205	0	0			375	911.415	911.79	0.375	SETOUT LEVEL TO MAXIMUM PIPE OBVERT
1A601-01	INTERALLOTMENT PIT 900x900	727525.717	6185272.534	0.9	0.9	375	910.755	375	910.495	912.025	1.531	
1A601-03	STUB CONNECTION	727524.236	6185266.027	0	0	375	910.423			910.798	0.281	SETOUT LEVEL TO MAXIMUM PIPE OBVERT
NOTE:		•		•		•						·
1. XY SETOUT TO PIT CENTRE												
2. SETOUT LEVEL TO PIT COVE	RLEVEL											
3. SOME SETOUT XY OR Z LEVE	LS HAVE SPECIAL SETOUT DATA. S	SEE INDIVIDUAL MA	ANHOLE REMARKS									

PIT				INTERNAL		INLET		OUTLET		PIT		
IAME	TYPE	EASTING	NORTHING	WD	LEN	DIA	INV LEV	DIA	INV LEV	SETOUT RL	DEPTH	REMARKS
A101-10	KERB INLET PIT - GRATE & 1.8m LINTEL - ON GRADE	727715.492	6185304.258	0.9	0.9			375	920.067	921.462	1.395	XY SETOUT TO SETOUT STRIN
A101-09	KERB INLET PIT - GRATE & 1.8m LINTEL - ON GRADE	727713.997	6185297.416	0.9	0.9	375	919.997	375	919.977	921.459	1.482	XY SETOUT TO SETOUT STRIN
A101-08	KERB INLET PIT - GRATE & 1.8m LINTEL - ON GRADE	727657.547	6185307.847	0.9	0.9	375	919.137	375	919.06	920.54	1.481	XY SETOUT TO SETOUT STRIN
A101-07	KERB INLET PIT - GRATE & 1.8m LINTEL - ON GRADE	727644.781	6185298.541	0.9	0.9	375	918.858	375	918.732	920.156	1.424	XY SETOUT TO SETOUT STRIN
A101-06	KERB INLET PIT - GRATE & 2.4m LINTEL - SAG	727637.197	6185255.695	0.9	0.9	375	918.06	375	918.04	919.476	1.436	XY SETOUT TO SETOUT STRIN
A101-05	KERB INLET PIT - GRATE & 2.4m LINTEL - SAG	727630.296	6185256.897	0.9	0.9	375	917.97	375	917.95	919.474	1.525	XY SETOUT TO SETOUT STRIN
A 101-04	KERB INLET PIT - GRATE & 1.8m LINTEL - ON GRADE	727615.823	6185250.271	0.9	0.9	375	917.79	375	917.77	919.503	1.733	
A101-03	KERB INLET PIT - GRATE & 1.8m LINTEL - ON GRADE	727568.95	6185258.102	0.9	0.9	375	915.522	375	915.437	916.895	1.458	
A601-03	KERB INLET PIT - GRATE & 1.8m LINTEL - ON GRADE	727524.136	6185265.588	0.9	0.9	375	910.855	375	909.815	912.232	2.416	
A101-01	EXISTING GRATED SURFACE INLET PIT	727486.156	6185271.913	0.9	0.9	375	907.89			908.962	1.072	
A102-01	KERB INLET PIT - GRATE & 1.8m LINTEL - ON GRADE	727659.116	6185314.676	0.9	0.9			375	919.15	920.545	1.395	XY SETOUT TO SETOUT STRIN
A101-08	KERB INLET PIT - GRATE & 1.8m LINTEL - ON GRADE	727657.547	6185307.847	0.9	0.9	375	919.08			920.54	1.481	XY SETOUT TO SETOUT STRIN
A601-11	INTERALLOTMENT PIT 900x900	727758.74	6185342.906	0.9	0.9			150	921.199	921.967	0.768	
A601-10	INTERALLOTMENT PIT 900x900	727739.093	6185346.558	0.9	0.9	150	920.825	225	920.729	921.575	0.846	
A601-09	INTERALLOTMENT PIT 900x900	727719.446	6185350.209	0.9	0.9	225	920.355	225	920.335	921.183	0.848	
A601-08	INTERALLOTMENT PIT 900x900	727699.799	6185353.861	0.9	0.9	225	919.845	225	919.82	920.67	0.85	
A601-07	INTERALLOTMENT PIT 900x900	727680.153	6185357.512	0.9	0.9	225	919.052	300	918.934	919.871	0.937	
A601-06	INTERALLOTMENT PIT 900x900	727660.506	6185361.164	0.9	0.9	300	918.168	300	918.126	919.069	0.943	
A601-05	INTERALLOTMENT PIT 900x900	727640.641	6185364.856	0.9	0.9	300	917.924	375	917.848	918.847	0.999	
2A601-04	INTERALLOTMENT PIT 900x900	727639.166	6185365.13	0.9	0.9	375	917.833	375	917.03	918.832	1.802	
A601-03	INTERALLOTMENT PIT 900x900	727645.601	6185398.688	0.9	0.9	375	914.765	375	914.745	916.529	1.784	
2A601-02	INTERALLOTMENT PIT 900x900	727589.735	6185361.081	0.9	0.9	375	913.351	375	913.299	914.603	1.304	
A601-01	INTERALLOTMENT PIT 900x900	727545.227	6185330.96	0.9	0.9	375	911.564	375	911.544	912.83	1.286	
A601-02	INTERALLOTMENT PIT 900x900	727541.778	6185320.632	0.9	0.9	375	911.435	375	911.415	912.813	1.398	
A601-01	EXISTING INTERALLOTMENT PIT	727525.717	6185272.534	0.9	0.9	375	910.755	375	910.495	912.025	1.531	
A601-03	KERB INLET PIT - GRATE & 1.8m LINTEL - ON GRADE	727524.136	6185265.588	0.9	0.9	375	910.423			912.232	2.416	

2. SETOUT LEVEL TO PIT COVER LEVEL
3. SOME SETOUT XY OR Z LEVELS HAVE SPECIAL SETOUT DATA. SEE INDIVIDUAL MANHOLE REMARKS

	REV	DESCRIPTION	DATE	DRAWN	DESIGNED	CHECKED	APPRVD	SCALE	GRID	DATUM	PROJECT MANAGER	CLIENT
3	C	MINOR AMENDMENTS	20/03/2025	SSH	PC	AVG	GT	0 5 10 15 20 25 30 35 40 45 50		mAHD	GT.	BLUE SOX DEVELOPMENTS
ASS.	В	MINOR AMENDMENTS	11/03/2025	SSH	PC	AVG	GT	A1 (A3) 1:500 (1:1,000) METRES		IIIAND	u i	DEOL SOX DEVELOT MENTS
₹	Α	INITIAL RELEASE	25/02/2025	SSH	PC	AVG	GT	0 1 2 3 4 5 6 7 8 9 10		& COPYRIGHT		PROJECT NAME/PLANSET TITLE
SE								A1 (A3) 1:100 (1:200) METRES	This plan must not principal certifying		on unless signed as approved by	PROPOSED 20 LOT SUBDIVISION
- :1										in millimetres unless of	therwise specified	
									This drawing must	not be reproduced in	whole or part without prior written	CONCEPT CIVIL ENGINEERING PLANS
륍									I .	s & Associates Pty Ltd		3 MEMORY AVE CROOKWELL NSW 2583
8									(C) Copyright I	Martens & Associa	tes Pty Ltd	

Martens & Environment Water Geotechnical Civil

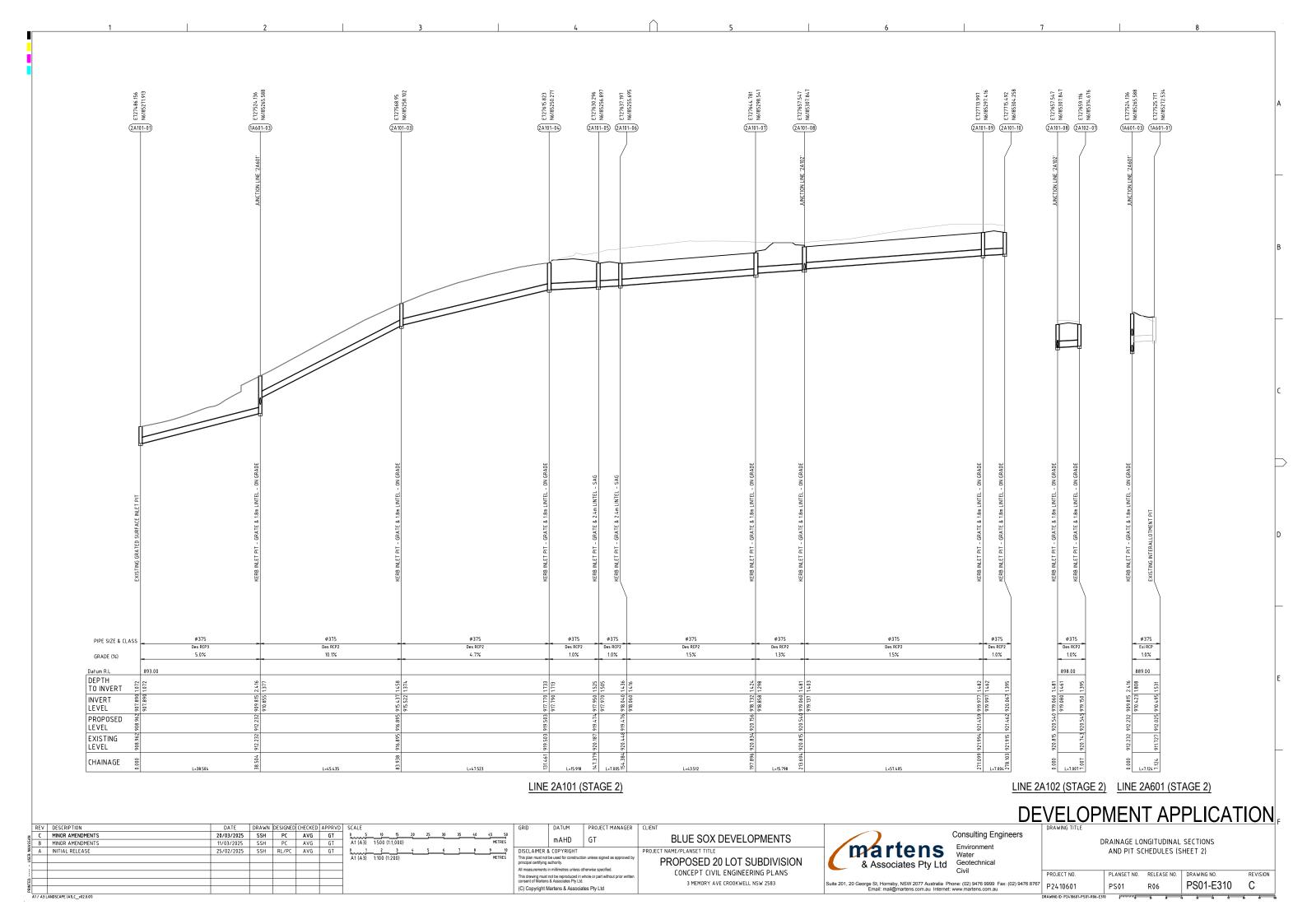
Consulting Engineers

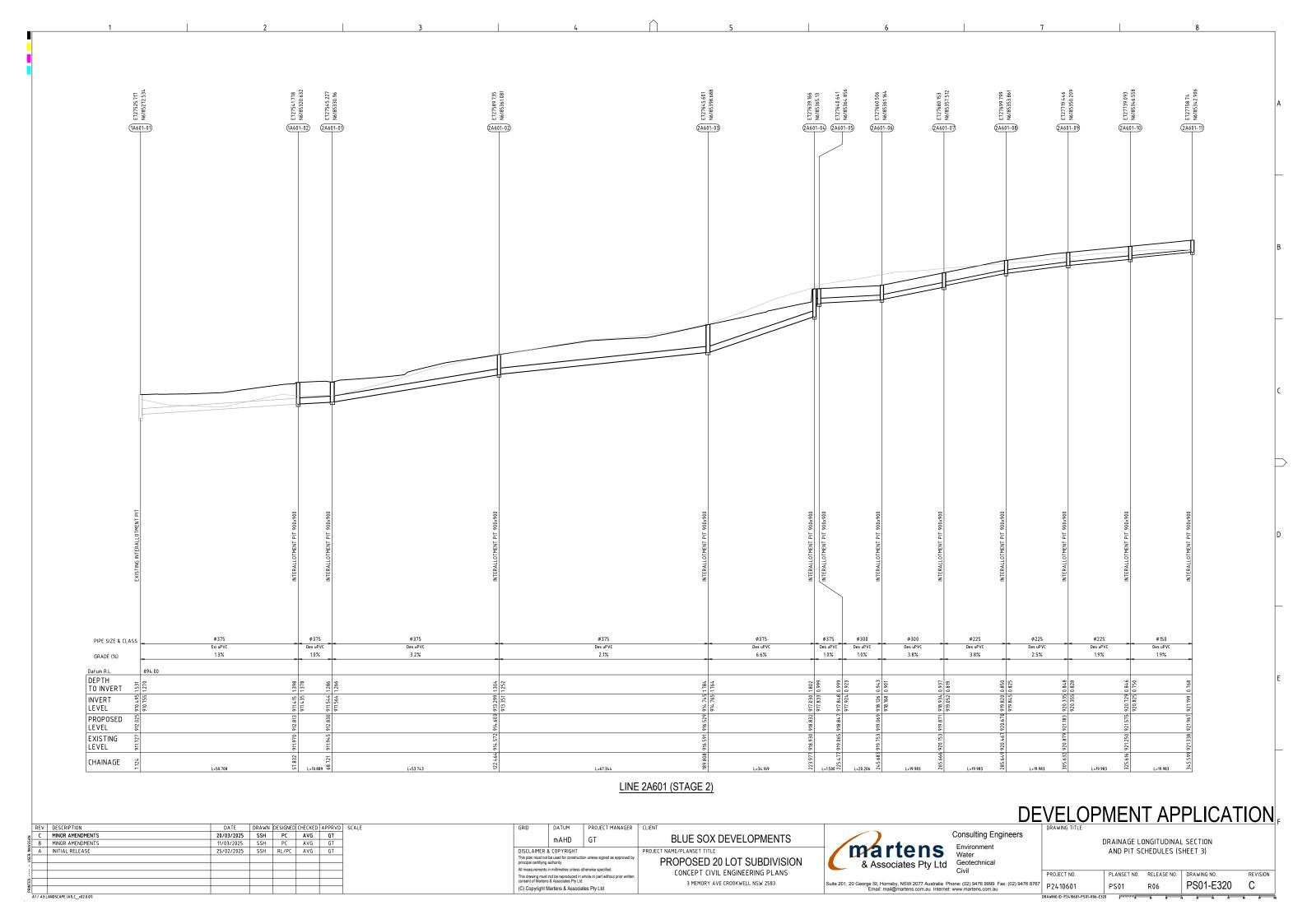
DRAINAGE LONGITUDINAL SECTIONS AND PIT SCHEDULES (SHEET 1) PLANSET NO. RELEASE NO. DRAWING NO.

DEVELOPMENT APPLICATION

Suite 201, 20 George St, Hornsby, NSW 2077 Australia Phone: (02) 9476 9999 Fax: (02) 9476 8767 Email: mail@martens.com.au Internet: www.martens.com.au

PROJECT NO. REVISION PS01-E300 С PS01 R06

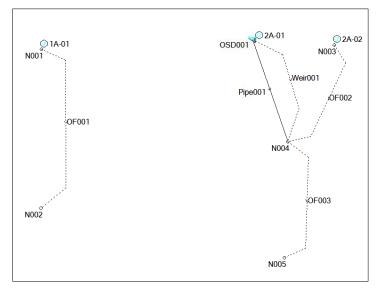




PRE DEVELOPMENT CATCHMENT DETAILS (P2410601DRN02V01)										
DRAINS CAT	DESCRIPTION	AREA (HA)	%PAVED							
1A-01	FUTURE LOT DEVELOPMENT AREA	0.080	0%							
	TOTAL AREA	0.080	100%	= % OF TOTAL AREA						
	TOTAL IMPERVIOUS AREA	0.000	0%	= % OF TOTAL AREA						
	TOTAL PERVIOUS AREA	0.080	100%	= % OF TOTAL AREA						

POST DEVELOPMENT CATCHMENT DETAILS (P2410601DRN02V01)										
DRAINS CAT	DESCRIPTION	AREA (HA)	%PAVED							
2A-01	FUTURE LOT - ROOF AREA	0.035	100%							
2A-02	FUTURE LOT OTHER IMPERVIOUS AREA	0.010	100%							
	FUTURE LOT LANDSCAPE	0.035	0%							
	TOTAL AREA	0.080	100%	= % OF TOTAL AREA						
	TOTAL IMPERVIOUS AREA	0.045	56%	= % OF TOTAL AREA						
	TOTAL PERVIOUS AREA	0.035	44%	= % OF TOTAL AREA						

	DRAINS MODELLING RESULTS (P2410601DRN02V01)									
STORM EVENT	PRE DEVELOPMENT FLOW RATE ((m3/sec)	POST DEVELOPMENT FLOW RATE ((m3/sec)	DIFFERENCE	(POST - PRE),COMPLIES (Y/N)						
0.2 EY	0.003	0.003	0.000	Y						
10% AEP	0.005	0.005	0.000	Y						
5% AEP	0.007	0.006	-0.001	Y						
2% AEP	0.01	0.009	-0.001	Y						
1% AEP	0.013	0.013	0.000	Y						



DRAINS MODELLING LAYOUT (P2410601DRN02V01)

- NOTES:

 1. OSD MODELLING HAS BEEN PERFORMED AT THE LOT LEVEL. NO OSD IS REQUIRED AT THE SUBDIVISION STAGE-ONLY WHEN PROPOSED LOTS ARE DEVELOPED. THE POST DEVELOPMENT CATCHMENT (BASED ON ANALYSIS OF RECENT NEIGHBOURING SUBDIVISIONS) FOR EACH PROPOSED LOT WAS ASSUMED TO HAVE:

 a. ROOF AREA OF 350m² (CONNECTED TO AN ABOVE GROUND OSD TANK)

 b. ADDITIONAL 100m² or Impervious AREA.

 c. REMAINING AREA CONSIDERED AS PERVIOUS.

 2. THE RESULTS SHOW THAT A 5 KL OSD TANK IS REQUIRED TO BE PROVIDED BY EACH LOT OWNER UPON DEVELOPMENT OF EACH LOT.

 DATE
 DRAWN
 DESIGNED
 CHECKED
 APPRVD
 SCALE

 20/03/2025
 SSH
 PC
 AVG
 GT

 11/03/2025
 SSH
 PC
 AVG
 GT

 25/02/2025
 SSH
 AVG
 AVG
 GT
 REV DESCRIPTION
C MINOR AMENDMENTS
B MINOR AMENDMENTS DATUM PROJECT MANAGER CLIENT BLUE SOX DEVELOPMENTS GT DISCLAIMER & COPYRIGHT PROJECT NAME/PLANSET TITLE INITIAL RELEASE This plan must not be used for construction unless signed as approprincipal certifying authority.

All measurements in millimetres unless otherwise specified. PROPOSED 20 LOT SUBDIVISION This drawing must not be reproduced in whole or part without prior written consent of Martens & Associates Pty Ltd.

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DEVELOPMENT APPLICATION OSD CATCHMENT PLAN, MODEL AND RESULTS PROJECT NO. PLANSET NO. RELEASE NO. DRAWING NO. REVISION PS01-E600 С PS01 R06

A1 / A3 LANDSCAPE (A1LC_v02.0.01)

